

5.2 Preliminary Geotechnical Site Investigation

5.2.1 Minimum requirements

5.2.1.1 The Competent Person (Geotechnics) shall, as a minimum, in order to satisfy the objectives of the preliminary Geotechnical Site Investigation stated in 4.1:

- a) approach the following organisations, as necessary, in order to gather data:
 - i) Mining Houses;
 - ii) the Council for Geoscience;
 - iii) the Deeds Office ;
 - iv) the Department of Land Affairs;
 - v) the Department of Water Affairs;
 - vi) the District Councils;
 - vii) the Government Mining Engineer;
 - viii) the Local Authority or regional databank of geotechnical data;
 - ix) the National Home Builders Registration Council;
 - x) the Surveyor General; and
 - xi) consultants;
- b) gather and assimilate available data pertaining to the site from the following sources, as necessary:
 - i) orthophotographic coverage (Scale 1:10 000);
 - ii) aerial photographic coverage;
 - iii) geological data and mapping concerning the site and immediate environs;
 - iv) topographic maps;
 - v) geohydrological data (regional and local in the case of dolomitic areas);
 - vi) mining data;
 - vii) geotechnical reports from surrounding developments, infrastructure and the like;
 - viii) geotechnical problems previously recorded in the area e.g. sinkholes in Dolomitic Areas, seismic activity and the like;
 - ix) any available regional geophysical data, such as regional gravity surveys, aeromagnetic surveys and the like; and
 - x) seismological data.
- c) review published geotechnical literature for the region;
- d) make appropriate enquiry to the office of Government Mining Engineer in regard to any and all land rezoning applications where release of land is provided in areas of acknowledged mining work;
- e) analyse data and identify and categorise terrain types in accordance with Table 3;
- f) verify terrain types in the field and examine all visible data, including ground profile exposures, and the results, of large scale ground excavation and or alteration by means of borrow pits, quarrying, mining, construction and related remedial works and rehabilitation that can be viewed and reasonably annotated on orthophotos or aerial photographs;
- g) indicate appropriate land uses;
- h) comment on potential sources of construction materials; and
- i) establish in principle whether the site is, or in future will be, influenced by underground or surface mining operations by making tentative enquiries to the Government Mining Engineer.

5.2.1.2 The Competent Person (Geotechnics) shall where the land ownership history includes a mining operator, provide a properly documented record or site plan in which the surface footprint of the mining or quarrying or material borrow area (opencast pit) or the (underground) mining plan is indicated on fully-coordinated drawings, based on the Government Mining Engineers records, on aerial photographs or ortho-mapping, as appropriate.

5.2.2 Reporting requirements

5.2.2.1 The Competent Person (Geotechnics) shall document and report all findings and Opinions in a written report using the following standard headings:

- Executive summary
- 1 Introduction
- 2 Information
 - 2.1 Description and list of the information assimilated and used in the study
 - 2.2 General location and description of site
 - 2.3 Evaluation procedures used in the investigations
 - 2.4 Geology and geohydrology of the site

- 2.5 Geotechnical conditions and constraints
- 2.6 Terrain mapping units
- 3 Impact of the geotechnical character of the site on subsidy housing developments
 - 3.1 Land usage
 - 3.2 Installation of services
 - 3.3 House construction
 - 3.4 Housing subsidy variations
- 4 Conclusions and recommendations.

Table 3: Geotechnical Classification for Urban Development (after Partridge, Wood and Brink)

CONSTRAINT		Most favourable (1)	Intermediate (2)	Least favourable (3)
A	Collapsible Soil	Any collapsible horizon or consecutive horizons totaling a depth of less than 750mm in thickness*	Any collapsible horizon or consecutive horizons with a depth of more than 750mm in thickness	A least favourable situation for this constraint does not occur
B	Seepage	Permanent or perched water table more than 1,5m below ground surface	Permanent or perched water table less than 1,5m below ground surface	Swamps and marshes
C	Active Soil	Low soil-heave potential anticipated*	Moderate soil heave potential anticipated	High soil-heave potential anticipated
D	Highly compressible soil	Low soil compressibility anticipated*	Moderate soil compressibility anticipated	High soil compressibility anticipated
E	Erodability of soil	Low	Intermediate	High
F	Difficulty of excavation to 1,5m depth	Scattered or occasional boulders less than 10% of the total volume	Rock or hardpan pedocretes between 10 and 40% of the total volume	Rock or hardpan pedocretes more than 40% of the total volume
G	Undermined ground	Undermining at a depth greater than 240m below surface (except where total extraction mining has not occurred)	Old undermined areas to a depth of 90-240 m below surface where slope closure has ceased	Mining within less than 90-240 m of surface or where total extraction mining has taken place
H	Stability: (Dolomite & Limestone)	Possibly stable. Areas of dolomite overlain by Karoo rocks or intruded by sills. Areas of Black Reef rocks. Anticipated Inherent Risk Class I	Potentially characterised by instability. Anticipated Inherent Risk Classes 2 – 5.	Known sinkholes and dolines. Anticipated Inherent Risk Classes 6 – 8.
I	Steep slopes	Between 2 and 6 degrees (all regions)	Slopes between 6 and 18 degrees and less than 2 degrees (Natal and Western Cape) Slopes between 6 and 12 degrees and less than 2 degrees (all other regions)	More than 18 degrees (Natal and Western Cape) More than 12 degrees (all other regions)
J	Areas of unstable natural slopes	Low risk	Intermediate risk	High risk (especially in areas subject to seismic activity)
K	Areas subject to seismic activity	10% probability of an event less than 100 cm/s ² within 50 years	Mining-induced seismic activity more 100 cm/s ²	Natural seismic activity more than 100 cm/s ²
L	Areas subject to flooding	A "most favourable" situation for this constraint does not occur	Areas adjacent to a known drainage channel or floodplain with slope less than 1%	Areas within a known drainage channel or floodplain

* These areas are designated as 1A, 1C, 1D, or 1F where localised occurrences of the constraint may arise.

Example: A sub-area designated as Zone 2BF would be an intermediate class with anticipated seepage and excavation problems.
A sub-area designated as Zone 3B would be designated as least favourable and not recommended for development due to surface water inundation.

5.2.2.2 The report must:

- a) include a discussion of the process followed to arrive at the Terrain Mapping Units as outlined and defined in Table 3;
- b) include a locality plan of the site complete with site boundaries, co-ordinates and property descriptions;
- c) indicate topographic and geological conditions clearly on appropriately co-ordinated and scaled maps with superimposed or overlaid property boundaries;
- d) discuss ground conditions in terms of the presence of outcrop, and likely cover soils, the origin of which may be initially interpreted from maps, aerial photos, orthophotos, available information and observations from the walk-over survey or inspection;
- e) provide a physical description of the surface soil condition, eg in alluvial floodplains, side gullies, undrained depressions or talus slopes, supported with photographic documentation of features of significance;

- f) contain appropriate comments on the presence of prominent water-courses and preferred drainage routes;
- g) present interpretations of groundwater seepage indications;
- h) contain comments on the structural conditions of any buildings or improvements on the land as an indicator of the influence of ground conditions; and
- i) include a drawing of the site showing terrain mapping units in accordance with the provisions of Table 3, complete with approximate co-ordinates.

5.2.2.3 The report and all drawings must also be available in an electronic format.

5.3. Phase 1 Geotechnical Site Investigations

5.3.1 Requirements for near surface soil horizons investigations

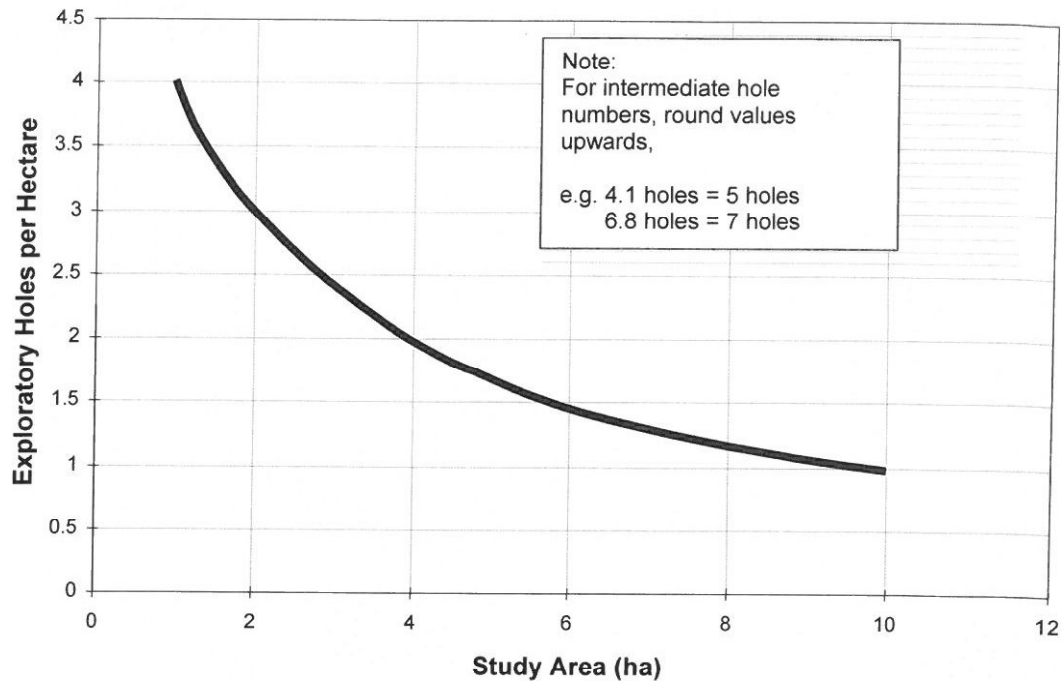
5.3.1.1 Minimum requirements

The Competent Person (Geotechnics) shall, as a minimum, in order to satisfy the objectives of the Phase 1 Geotechnical Site Investigation in non-Dolomitic Land, or areas underlain by dolomites where the risk of sinkhole and doline formation is acceptable, as stated in 4.2:

- a) conduct a detailed Geotechnical Site Investigation involving an in-situ evaluation of the ground profile to a minimum depth of 3,0 m or to the machine refusal depth at a frequency derived from Figures 1a and 1b, representative sampling, laboratory testing and the analysis of physical and (basic) chemical properties of all representative soil horizons which can be expected to influence improvements to the land relating to the subsidy housing development at the frequency derived from Table 4, including:
 - i) foundations and structural nature of residential housing;
 - ii) construction of roads (surfaced and gravel);
 - iii) excavations for and construction of buried services including appropriate trench backfills;
 - iv) landslip (slope instability); and
 - v) present and past mining activities;
- b) judge, in mine related land, the long-term prognosis for excessive settlement and particularly differential settlement which has the potential to give rise to unacceptable development due to factors such as:
 - i) water-bearing service disruptions arising from loss of positive gradients, rupture due to ground settlement;
 - ii) loss of positive stormwater run-off from zones of substantial settlement and resulting flooding, infiltration and exacerbated water-induced settlement;
 - iii) loss of serviceability in structures due to rotation / tilt or settlement even where structural distress is controlled by adequate foundation stiffness; and
 - iv) restrictions that will inevitably be placed on housing development to mitigate the negative impacts of the settlement process;
- c) prepare a comprehensive geotechnical report which:
 - i) provides the township description and defines the extent and boundaries of the township;
 - ii) describes the local geology;
 - iii) describes soil profile by Site Classification unit;
 - iv) provides geotechnical interpretation of each soil profile unit;
 - v) provides a provisional Site Classifications of the site in accordance with Table 5;
 - vi) contains foundation recommendations by Site Classification unit;
 - vii) contains earthworks (materials) and excavation classifications;
 - viii) assesses the stability and related (geotechnical) parameters;
 - ix) identifies conditions and constraints such as mining related problems, areas of outcrop, slope instability, contaminated land, unconsolidated fill etc; and
 - x) provides information on the drainage of the site.

Note: The minimum extent of fieldwork and laboratory testing required is intended to give a minimum requirement of input data on the basis of which realistic engineering judgements may be made and site classification boundaries drawn, which provide the developer with a realistic sampling of the ground conditions. Clearly, where complex geological and topographic conditions exist eg in mine related land, uncontrolled fills (made up ground without controlled compaction), unstable slopes etc, or where the soils are highly Variable, supplementary work can be anticipated requiring additional input, deeper drilling or subsequent stages of investigation.

1a: Minimum frequency of exploratory holes in near surface soil horizons where the site is not greater than 10ha



1b: Minimum frequency of exploratory holes in near surface soil horizons where site is greater than 10ha

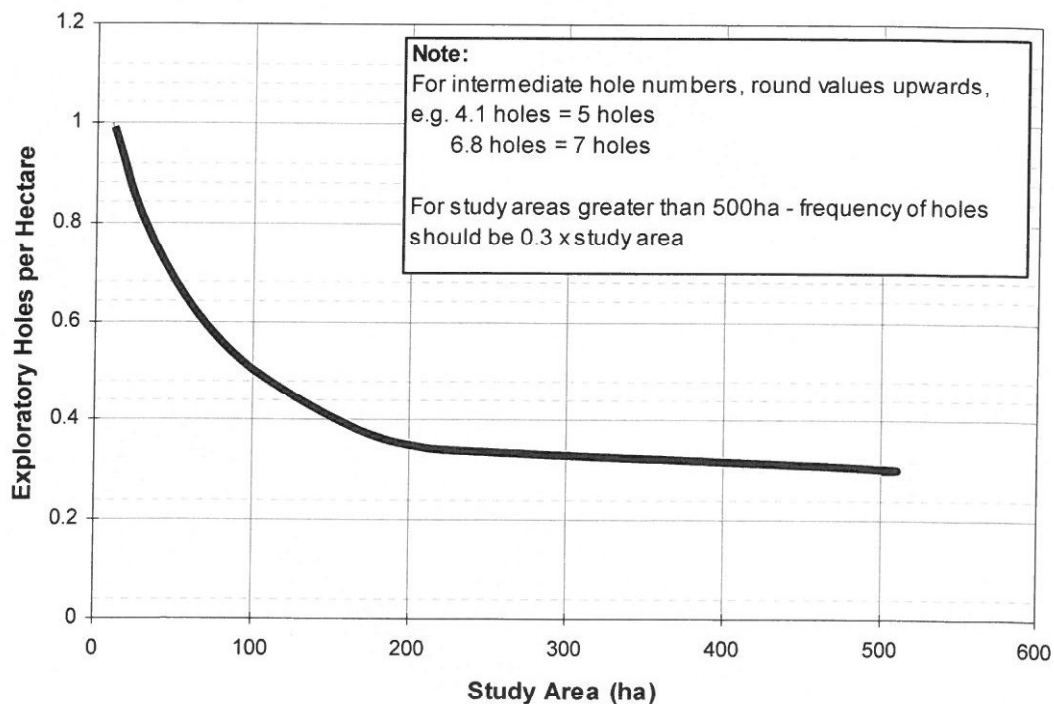


Figure 1: Minimum frequency of exploratory holes in near surface soil horizons

Table 4: Minimum Quantities of Laboratory Testing for Different Sizes of Study Areas

Study Area (ha)	Foundation Indicator / MC	Consolidometer/ Swell	Chemistry
<5	3	2	2
5 to 10	4	3	2
11 to 20	6	4	3
21 to 50	10	5	5
51 to 100	15	6	10
101 to 200	20	10	10
201 to 500	50	20	10

5.3.1.2 Field work requirements

5.3.1.2.1 Exploratory holes shall be set out using a hand held GPS, coordinated by survey or position using locally referenced landmarks or survey pegs.

5.3.1.2.2 Fieldwork shall consist of the excavation of the exploratory holes, in-situ profiling or sampling holes in a fashion appropriate to the ground conditions, groundwater, sampling requirement, personnel safety considerations and the like, using one or more of the following methods:

- TLB or excavator pits or trenches;
- penetrometer probes (continuous standard penetrometer test and / or dynamic probe super heavy penetrometer tests) with sample recovery;
- large diameter (not less than 750mm) auger holes;
- hand held DCP's supplemented by methods a) and c);
- hand dug pits supplemented by other methods;
- percussion drilling with sample recovery supplemented by other methods; and
- rotary core drilling with sample recovery.

5.3.1.2.3 All In-situ Soil Profiling shall be in accordance with the provisions Jennings, Brink and Williams (1973) procedure and the Guidelines for Soil and Rock Logging. The Soil Profile shall record and be described in the following terms for each individual soil horizon exposed in the exploratory holes:

- moisture;
- colour;
- consistency;
- structure;
- soil type;
- origin; and
- any other detail relevant to the engineering assessment of the in-situ soil conditions.

5.3.1.2.4 Zones of seepage and presence of water tables must be recorded, as well as the nature of the water table, ie perched or permanent.

5.3.1.2.5 The Competent Person (Geotechnics) shall take reasonable steps to ensure that all the provisions of the Code of Practice on the Safety of Men Working in Small Diameter Shafts and Test Pits for Civil Engineering Purposes are observed.

5.3.1.3 Laboratory testing requirements

The Competent Person (Geotechnics) shall arrange for the following laboratory test procedures to be undertaken by an accredited soil mechanics test laboratory on representative bulk, disturbed and / or undisturbed samples of all significant ground profile variants, in order to provide Interpretative Data for judging ground response to foundations, earthworks construction, excavation stability, chemical aggressiveness towards buried services and the like:

- particle size distribution / grading;
- Atterberg Limits;
- Moisture content;
- Compressibility / potential collapse;
- Swell under load;
- pH and conductivity;
- Compaction (moisture : density relationship); and

h) CBR.

Table 5: Residential site class designations (After Watermeyer and Tromp (1992) and the Joint Structural Division)

TYPICAL FOUNDING MATERIAL	CHARACTER OF FOUNDING MATERIAL	EXPECTED RANGE OF TOTAL SOIL MOVEMENTS (mm)	ASSUMED DIFFERENTIAL MOVEMENT (% OF TOTAL)	SITE CLASS
Rock (excluding mud rocks which may exhibit swelling to some depth)	STABLE	NEGLIGIBLE	-	R
Fine grained soils with moderate to very high plasticity (clays, silty clays, clayey silts and sandy clays)	EXPANSIVE SOILS	< 7,5 7,5 – 15 15 – 30 > 30	50% 50% 50% 50%	H H1 H2 H3
Silty sands, sands, sandy and gravelly soils	COMPRESSIBLE AND POTENTIALLY COLLAPSABLE SOILS	<5 5-10 >10	75% 75% 75%	C C1 C2
Fine grained soils (clayey silts and clayey sands of low plasticity), sands, sandy and gravelly soils	COMPRESSIBLE SOILS	<10 10-20 >20	50% 50% 50%	S S1 S2
Contaminated soils, Controlled fill, Dolomitic areas, Landslip, Landfill, Marshy areas Mine waste fill, mining subsidence Reclaimed areas, Uncontrolled fill, Very soft silts/silty clays	VARIABLE	VARIABLE		P

NOTE:

- 1) The classifications, C, H, R and S are not intended for dolomitic areas unless specific investigations are carried out to assess the stability (risk of sinkholes and doline formation) of the dolomites. Where the risk is found to be acceptable, the site shall be designated in accordance with note 10.
- 2) Site classes are based on the assumption that differential movements, experienced by single-storey residential structures, expressed as a percentage of the total soil movements are approximately equal to 50% for soils that exhibit expansive or compressive characteristics and 75% for soils that exhibit both compressible and collapse characteristics. Where this assumption is incorrect or inappropriate, the total soil movements must be adjusted so that the resultant differential movement implied by the Table is equal to that which is expected in the field.
- 3) In some instances, it may be more appropriate to use a composite description to describe a site more fully e.g., C1/H2 or S1 and/or H2. Composite site classes may lead to higher differential movements and result in design solutions appropriate to a higher range of differential movement e.g., a Class R/S1 may be described as a Class S2 site. Alternatively, a further site investigation may be necessary as the final design solution may depend on the location of the housing unit on a particular site.
- 4) Where it is not possible to provide a single site designation and a composite description is inappropriate, sites may be given multiple descriptions to indicate the range of possible conditions, e.g. H1-H2 or C1-C2.
- 5) Soft silts and clays usually exhibit high consolidation and low bearing characteristics. Structures founded on these horizons may experience high settlements and such sites should be designated as Class S1 or S2, as relevant and appropriate.
- 6) Sites containing contaminated soils include those associated with reclaimed mine land; land down slope of mine tailings and old land fills.
- 7) Where a site is classified as being P, full particulars relating to the founding conditions on the site must be provided.
- 8) Where sites are designated as being Class P; the reason for such classification shall be placed in brackets immediately after the suffix, i.e. P (contaminated soils). Under certain circumstances composite description may be appropriate.
- 9) Certain fills may contain contaminants, which present a health risk. The nature of such fills should be evaluated and should be clearly demarcated as such.
- 10) Dolomitic Areas should be designated as being Class P (Dolomites-D2/H2) or Class P (Limestones- D2/H2) where the first designation after dolomites / limestones is the designation obtained from Table 6 and the second designation is from Table 5.

5.3.1.4 Reporting requirements

5.3.1.4.1 The Competent Person (Geotechnics) shall document and report all findings and Opinions in a written report using the following standard headings:

- Executive summary
- 1 Introduction and Terms of Reference
- 2 Information used in the study
- 3 Site Description
- 4 Nature of Investigation
- 5 Site Geology and Groundwater Conditions
 - 5.1 General
 - 5.2 Soil Profile
 - 5.3 Water Table
- 6 Geotechnical Evaluation
 - 6.1 Engineering and Material Characteristics

6.2	Slope Stability and Erosion
6.3	Excavation Classification with respect to Services
6.4	Impact of the Geotechnical Character of the Site on Subsidy Housing Developments
7	Site Classification
8	Foundation Recommendations and Solutions
9	Drainage
10	Special Precautionary Measures
11	Conclusions
	Appendices

5.3.1.4.2 The report must:

- a) describe and list the information assimilated and used in the study in Section 2 (Information used in the study);
- b) provide particulars of site boundaries and a description of the property in Section 3 (Site Description);
- c) describe the field investigation procedures used and laboratory tests undertaken in Section 4 (Nature of Investigation);
- d) state engineering and material characteristics which will affect the development and construction including the identification of conditions and constraints such as mining related problems, areas of outcrop, slope instability, contaminated land, unconsolidated fill, etc in Subsection 6.1 (Engineering and Material Characteristics);
- e) evaluate and establish the potential for lateral soil movement arising from surface erosion, soil creep, talus movement and slope instability in Subsection 6.2 (Slope Stability and Erosion);
- f) establish, for the purposes of broadly estimating subsidy variations (see Annexure 1) the presence and extent of:
 - i) permanent or perched water table less than 1,0 below ground surface;
 - ii) permanent or perched water table less than 1,5 below ground surface;
 - iii) the Unified Soil Classification of the uppermost soil horizon (0 to 750mm) where the average slope is steeper than or equal to 1:7,5; and
 - iv) the presence of hard rock and / or boulder class excavation (see Annexure 2) in future service trenches up to a depth of 1,5 metres;
- g) assess the suitability of the material in accordance with Annexure 2 in the upper 1,5 m of the site for excavation by hand in Subsection 6.3 (Excavation Classification with respect to Services);
- h) discuss foundation recommendations in relation to Watermeyer and Tromp (1992) and the Joint Structural Division's Code of Practice) and provide geotechnical engineering data associated with the design of such foundations in Section 8 (Foundation Recommendations and Solutions);
- i) discuss the effect of both surface water (flooding and ponding) and groundwater (marshy conditions, underground erosion, hydrostatic pressure and fluctuating water levels on the development and comment on whether or not the groundwater will be potentially harmful with respect to buried concrete and steel in Section 9 (Drainage);
- j) contain all soil profiles and the results of laboratory and in situ field tests including penetrometer test results in an orderly manner in the Appendices;
- k) include the following drawings:
 - i) a locality plan of site;
 - ii) a site plan showing positions of exploratory holes; and
 - iii) a soil map defining approximate boundaries of areas with common Site Class designations.

5.3.1.4.3 The Competent Person (Geotechnics) shall, where the land ownership history includes a mining operator, provide in the report details of depths of shallowest mining, backfill method and materials, the Government Mining Engineer's requirements and / or conditions of future land use and / or development and any investigation studies required to proceed with housing development.

5.3.1.4.4 Drawings shall be to a common scale, legible and easily reviewed. All drawings shall be correctly referenced with a clear indication of co-ordinates.

5.3.1.4.5 The report and all drawings must also be available in an electronic format.

5.3.2 Requirements for stability investigations in Dolomitic Areas

5.3.2.1 Minimum requirements

The Competent Person (Geotechnics) shall, as a minimum in order to satisfy the general requirements for a Phase 1 Geotechnical Site Investigations in Dolomitic Areas as stated in 4.2:

- a) map the basic geology and geomorphological features of the site as well as any sinkholes and dolines within or in close proximity to the site;
- b) formulate an Opinion as to risk characterisation and land use of the site in terms of Table 1, using geophysics, the assessment of the morphology, subsurface profile from ground surface to dolomite bedrock and the geohydrological regime conditions and groundwater compartmentalization;
- c) conduct a detailed Geotechnical Site Investigation comprising a gravity survey in accordance with the provisions of 5.3.2.2 and drilling in accordance with the provisions of 5.3.2.3 to a minimum depth of 6,0 into the bedrock at a frequency derived from Figures 2a and 2b;
- d) prepare a comprehensive geotechnical report which:
 - i) provides the township description and defines the extent and boundaries of the township;
 - ii) establishes the geological changes over the site;
 - iii) establishes the nature, fluctuations, compartmentalisation, and original ground water levels from geohydrological Data;
 - iv) describes and interprets the local geology by Site Classification Unit;
 - vi) provides appropriate land use proposals by Site Classification unit in accordance with Table 1;
 - v) provides a Site Classifications of the site in accordance with Table 6;
 - vi) contains recommendations for geotechnical and structural solutions in sites designated as D3 by Site Classification unit with reference to Wagener and Buttrick, Van Schalkwyk, Kleywegt and Watermeyer (2002);
 - x) presents appropriate water precautionary measures;
 - xi) identifies precautionary measures in addition to the mandatory measures contained in the Home Building Manual; and.
 - xii) outlines an appropriate Risk Management Plan.

Note: Multiple remote sensing techniques in addition to the mandatory gravity survey may be required, depending on the geology, e.g. electro magnetics to establish the contact between dolomite and conductive material such as Karoo shale.

Table 6: Dolomitic Area designations (After Joint Structural Division and Buttrick, Van Schalkwyk; Kleywegt and Watermeyer, 2001)

Dolomitic Area Class	Description
D1	No precautionary measures are required to permit the construction of housing units due to an adequate overburden thickness.
D2	The risk of sinkhole and doline formation is adjudged to be such that only general precautionary measures, which are intended to prevent the concentrated ingress of water into the ground, are required to permit the construction of housing units.
D3	The risk of sinkhole and doline formation is adjudged to be such that precautionary measures in addition to those pertaining to the prevention of concentrated ingress of water into the ground, are required to permit the construction of housing units.
D4	The risk of sinkhole and doline formation is such that precautionary measures cannot adequately reduce such risks to acceptable limits so as to permit the construction of housing units or the precautionary measures which are required are impracticable to implement.

5.3.2.2 Gravity survey requirements

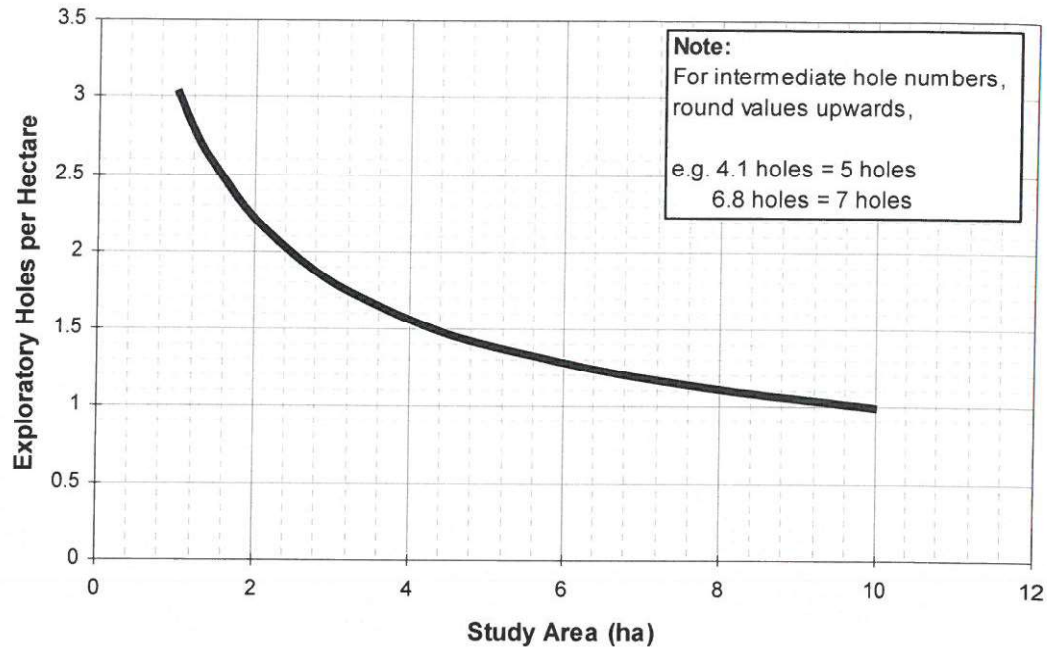
5.3.2.2.1 The gravity survey shall be undertaken by a suitably qualified geophysicist, with at least 3 years total working experience in dolomite environments of South Africa. Observations may be undertaken by a suitably qualified and experienced geotechnician under the direction of an experienced geophysicist.

5.3.2.2.2 The grid spacing for gravity surveys shall not exceed the lesser of 30 metres and the anticipated thickness of the overburden above the dolomites or limestones.

5.3.2.2.3 Five to ten percent of observations shall be repeated for control purposes.

5.3.2.2.4 A residual gravity map shall be produced and utilised to determine initial borehole positions. After an initial phase of drilling, a provisional residual gravity map shall be produced. The final residual gravity map shall only be produced after drilling is completed.

2a: Minimum frequency of percussion boreholes in Dolomitic Areas for study areas not greater than 10ha



2b: Minimum frequency of percussion boreholes in Dolomitic Areas for study areas greater than 10ha

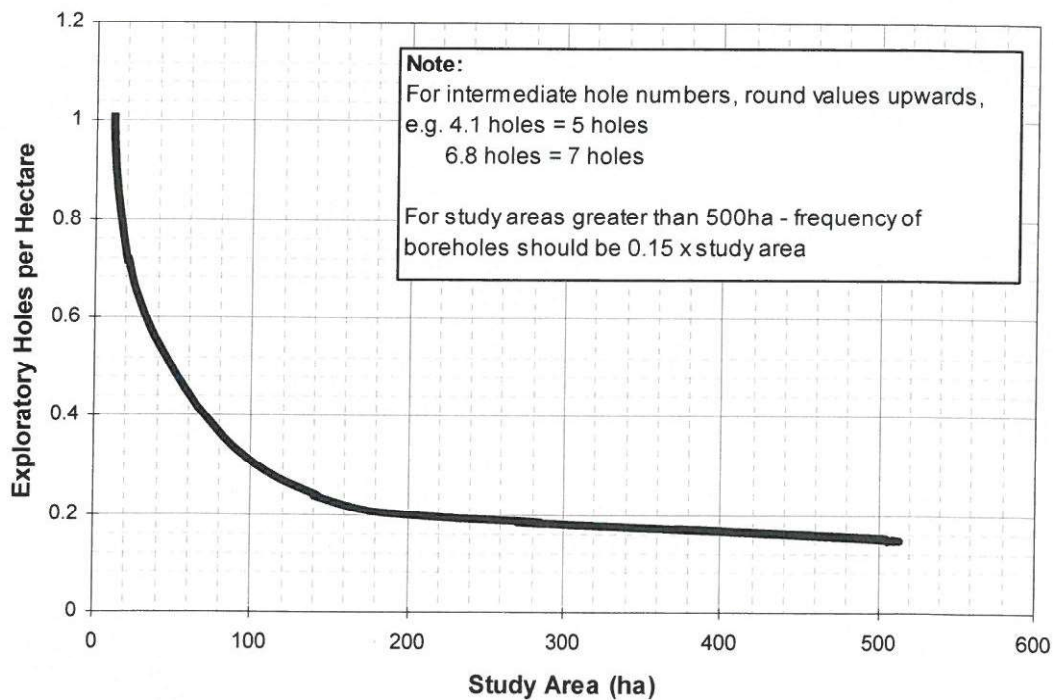


Figure 2: Minimum frequency of percussion boreholes in Dolomitic Areas

5.3.2.2.5 The accuracy of reduced observations on a relative basis shall be at least 0,01 mgal or better.

5.3.2.2.6 Contour intervals of not more than 0.1 milligals are to be used.

5.3.2.2.7 A geophysical report shall be produced describing the work procedures, interpretation and conclusions of the survey.

5.3.2.3 Drilling work requirements

5.3.2.3.1 Rotary percussion boreholes shall be drilled on the site utilising geophysical data to select the positions. Retrieve samples for every 1m drilled shall be retrieved.

5.3.2.3.2 Boreholes shall be drilled at least 6m into solid dolomite or limestone bedrock.

5.3.2.4.3 Drilling equipment shall comprise of the following mobile units:

- a) Compressor unit with measured and calibrated constant air delivery rating at 750 cfm and 16 Bar minimum.
- b) Pneumatic percussion drilling rig with 165mm nominal diameter button bit capable of drilling in all soil and rock types.

5.3.2.3.4 The following minimum information shall be recorded on the Drilling Sheet during the drilling of each borehole:

- a) Driller/drilling contractor;
- b) Date of drilling borehole;
- c) Drilling rig;
- d) Compressor type, capacity and delivery;
- e) Hammer size;
- f) Depth intervals for sampling (1m);
- g) Penetration times;
- h) Formation e.g. cavity, very soft, soft, reasonably hard, hard, solid;
- i) Hammer tempo e.g. highly irregular, irregular, regular;
- j) Air loss e.g. none, partial, total;
- k) Moisture condition e.g. water intercepted, wet, moist, dry;
- l) Borehole raveling/collapsing;
- m) Water or foam added;
- n) Casing used;
- o) Sample retrieval e.g. good, medium, poor, none; and
- p) Other remarks or comments.

5.3.2.3.5 The depth range of use of water or foam to enhance sample recovery shall be clearly indicated on the drillers field report.

5.3.2.3.6 Boreholes shall be backfilled with soil, recovered from drilling, suitably moistened to form a flowing slurry. The borehole shall be capped using a 400mm x 400mm x 150mm 15MPa wood floated concrete cap with 100mm concrete down the hole. The borehole number, drilling date and direction of inclination shall be marked in the wet concrete.

5.3.2.4 Requirements for descriptions of borehole chip sample

5.3.2.4.1 The method of borehole chip sample description shall be in accordance with the published recommendations of the South African Institution of Engineering and environmental Geologists.

5.3.2.4.2 Descriptions of borehole chip samples recovered shall include the following:

- a) Soil component:
 - i) Colour
 - ii) Soil type
- b) Rock component:
 - i) Chip shape e.g. angular, sub angular, sub rounded, rounded, etc.
 - ii) Colour
 - iii) Weathering described in terms of unweathered, slightly, medium, highly and completely weathered.
 - iv) Rock type.

5.3.2.4.3 The major portion of the sample is to be described first followed by the description of the sub-ordinate material. Subordinate portions are to be described using descriptions outlined below:

- i) Trace.
- ii) Minor.
- iii) Abundant.
- iv) Equal amounts.

5.3.2.4.4 The name of the driller, date of drilling, date of logging, loggers name, compressor capacity and delivery, drill rig type, hammer size, penetration times, level at which water is struck, groundwater rest level, ground elevation and co-ordinates shall be recoded on the profile sheet together with any problematic conditions such as air loss, sample loss and cavitation.

5.3.2.5 Requirements for gathering of geohydrological Data

5.3.2.5.1 The available geohydrological data shall be obtained from the Department of Water Affairs.

5.3.2.5.2 Where groundwater is encountered in a borehole, the level shall be established at least 24 hours after completion of the borehole. Two readings shall be taken within a week thereafter.

5.3.2.6 Requirements for the determination of Hazard and Risk Zones

Hazard, Inherent Risk and Development Risk shall be determined in accordance with the procedure outlined by Buttrick, Van Schalkwyk, Kleywegt and Watermeyer (2001) as summarized in Annexure 3.

5.3.2.7 Reporting requirements

5.3.2.7.1 The Competent Person (Geotechnics) shall document and report all findings and Opinions in a written report using the following standard headings:

- Executive summary
- 1 Introduction and Terms of Reference
- 2 Information used in the study
- 3 Site Description
- 4 Nature of Investigation
- 5 Site Geology and Groundwater Conditions
 - 5.1 General
 - 5.2 Profile
 - 5.3 Ground water levels
- 6 Stability Evaluation
 - 6.1 Dolomite stability characterisation
 - 6.2 Characterisation procedure.
 - 6.3 Stability characterisation of the site.
- 7 Conclusions and Recommendations
 - 7.1 Summary of Risk Zonation
 - 7.2 Appropriate land use recommendations
 - 7.3 Water and foundation precautionary measures
 - 7.4 Development densities and types
 - 7.4 Dolomite Area Designations for each Inherent Risk Zone.
 - 7.5 Specific founding measures in D3 areas.
 - 7.7 Outline of the preliminary Risk Management Plan.
- Appendices

5.3.2.7.2 The report must:

- a) describe and list the information assimilated and used in the study in Section 2 (Information used in the study);
- b) provide particulars of site boundaries and a description of the property in Section 3 (Site Description);
- c) describe the field investigation procedures used and laboratory tests undertaken in Section 4 (Nature of Investigation);
- d) discuss the geology and geohydrology in both the regional and site specific context in Section 5 (Site Geology and Groundwater Conditions);

- e) explain and motive the risk characterisation of the site in Section 6 (Stability Evaluation);
- f) specify water precautionary and special founding measures for each risk zone in Subsection 7.3 (Water and foundation precautionary measures);
- g) assess the impact of the Site Classes on subsidy housing developments;
- h) discuss foundation recommendations in D3 zones in relation to Wagener (1985) and Buttrick, van Schalkwyk, Kleywegt and Watermeyer (2002) and provide geotechnical engineering data associated with the design of such foundations in Subsection 7.5 (Specific founding measures in D3 areas);
- i) contain all Data from the gravity survey, boreholes and the results of laboratory and in situ field tests in an orderly manner in the Appendices;
- j) include the following drawings:
 - i) Locality plan of site.
 - iii) Site plan showing positions of boreholes and gravity contours.
 - iii) Zone map defining approximate boundaries of areas with common Site Class designations.

5.3.2.7.3 Drawings shall be to a common and appropriate scale, legible and easily reviewed. All drawings shall be correctly referenced with a clear indication of co-ordinates.

5.3.2.7.4 The report and all drawings must also be available in an electronic format.

5.4 Phase 2 Geotechnical Site Investigations

5.4.1 Requirements for Non-Dolomitic Areas

5.4.1.1 Minimum requirements

The Competent Person (Geotechnics) shall, as a minimum in order to satisfy the general requirements for a Phase 2 Geotechnical Site Investigations non-Dolomitic Areas as stated in 4.3:

- a) established formal profiling procedures with the person responsible for the installation of township services so that the available trenching is optionally utilized within the construction framework and programme for profiling purposes;
- b) co-ordinate activities associated with the profiling procedures;
- c) observe and record soil profiles in exposed services trenches at not more than 100m intervals or wherever soil type changes occur;
- d) undertake where justified supplementary Geotechnical Site Investigations;
- e) arrange for undisturbed samples to be taken for a set of Foundation Indicator Tests at a frequency of not more than one set for every five points profiled; and
- f) record data on field sheets and the points in the trenches which were profiled on a site layout plan.

5.4.1.2 Reporting

5.4.1.2.1 The Competent Person (Geotechnics) shall prepare a brief report as an addendum to the Phase 1 report which shall contain:

- a) a drawing indicating the location of the points profiled in the service trenches;
- b) records of all profiles and tests; and
- c) a marked up township layout drawing which confirms the Site Classes of each individual erf

5.4.1.2.2 The report and all drawings must also be available in an electronic format.

5.4.2 Requirements for Dolomitic Areas

5.4.2.1 Minimum requirements

The Competent Person (Geotechnics) shall satisfy the requirements of 5.4.1 and, as a minimum in order to satisfy the general requirements for a Phase 2 Geotechnical Site Investigations Dolomitic Areas as stated in 4.3:

- a) interact with the town planners, civil engineers and the developer concerning appropriate planning, design of infrastructure and housing units;
- b) develop a Risk Management Plan specific to the development (refer to Annexure 3);
- c) establish formal inspection procedures with the person responsible for the installation of township services so that the available trenching is optimally utilized within the construction framework and programme for inspection purposes;

- d) co-ordinate activities associated with the inspection procedures;
- e) inspect the service trenches to verify the Phase 1 stability zonation and to check for paleofeatures;
- f) investigate potential paleosinkhole structures wherever they are exposed; and
- g) undertake where justified supplementary Geotechnical Site Investigations.

5.4.2.2 Reporting requirements

5.4.2.2.1 The Competent Person (Geotechnics) shall prepare a brief report as an addendum to the Phase 1 report which shall contain:

- a) a drawing indicating the location of the points profiled in the service trenches;
- b) records of all profiles and tests; and
- c) a township layout drawing which confirms the Site Classes of each individual erf;

5.4.2.2.2 The report and all drawings must also be available in an electronic format.

Annexure 1:
Schedule of generic subsidy variations for site and founding conditions

CATEGORY OF SUBSIDY VARIATION	VERIFICATION CRITERIA	FACTORS TO CONSIDER IN ESTABLISHING QUANTUM OF SUBSIDY VARIATION
II: SITE CONDITIONS		
1 Seepage / ground water		
1.1 Category 1	Permanent or perched water table less than 1,0 m below ground surface.	Subsurface drainage / improved dampproofing measures to houses; service trenches to be dewatered during construction.
1.2 Category 2	Permanent or perched water table less than 1,5 m below ground surface.	Service trenches to be dewatered during construction.
2 Erodability of soil	Uppermost soil horizon (0 to 750mm) over erf is classified in terms of the Unified Soil Classification as SP, SM, CL or CH and average slope of erf measured in any direction is steeper than or equal to 1:7,5.	Provision of retaining wall and earthworks required to reduce slopes.
3 Difficulty of servicing of land due to slopes		
3.1 Type 1 site	Average slope measured along a 100 metre line in any direction from any of the boundaries of the erf is flatter than 1:100.	Difficulties associated the provision of waterborne sanitation and the drainage of sites/ provision of pump stations.
3.2 Type 2 site	Average slope measured across the erf in any direction exceeds 1:20 but is flatter than or equal to 1:10.	Terracing for houses/ additional masonry units in foundation walls required.
3.3 Type 3 site	Average slope measured across the erf in any direction exceeds 1:10 but is flatter than or equal to 1:7,5.	Terracing for houses required. Additional earthworks to roads and storm water control measures
3.4 Type 4 site	Average slope measured across the erf in any direction exceeds 1:7,5 but is flatter than or equal to 1:5.	Terracing for houses required. Additional earthworks to roads and storm water control measures.
3.5 Type 5 site	Average slope measured across the erf in any direction exceeds 1:5.	Terracing for houses required. Additional earthworks to roads and storm water control measures.
4 Difficulty of excavation		
4.1 Type 1 condition	Average slope measured across the erf in any direction is flatter than or equal to 1:10 and between 10 and 40% of material to a depth of 1,5 metres below pre-development level is classified as hard rock excavation.	Additional cost of trench excavation.
4.2 Type 2 condition	Average slope measured across the erf in any direction is flatter than or equal to 1:10 and in excess of 40% of material to a depth of 1,5 metres below pre-development level is classified as hard rock excavation.	Additional cost of trench excavation.
4.3 Type 3 condition	Average slope measured across the erf in any direction is steeper than 1:10 and the material to a depth of 1,5 metres below pre-development level is classified as Boulder Class B excavation.	Additional cost of trench excavation. Additional cost of road excavation.
4.4 Type 4 condition	Average slope measured across the erf in any direction is steeper than 1:10 and the material to a depth of 1,5 metres below pre-development level is classified as Boulder Class A or Hard Rock excavation.	Additional cost of trench excavation. Additional cost of road excavation.
5 Precautionary measures in sites underlain by dolomites/ limestones	Class P(dolomites/limestones- D2/D3) site class designation in accordance with 2.5 and 2.8 of Part 1 of Section 2 of the NHBRC Home Building Manual.	The design and construction of township services will need to be in accordance with 2.8.3 of Part 1 of Section 2 of the NHBRC Home Building Manual.

CATEGORY OF SUBSIDY VARIATION	VERIFICATION CRITERIA	FACTORS TO CONSIDER IN ESTABLISHING QUANTUM OF SUBSIDY VARIATION
III: FOUNDING CONDITIONS# 1 Expansive soils 1.1 Class H1 1.2 Class H2 1.3 Class H3 2 Compressible and potentially collapsible soil 2.1 Class C1 2.2 Class C2 3 Compressible soils 3.1 Class S1 3.2 Class S2 4 Variable 4.1 Class P (Dolomites /Limestones– D2) 4.2 Class P (Dolomites / Limestones– D3) 4.3 Class P (Mining subsidence)	<p>Site class designations classified in accordance with 2.5 of Part 1 Section 2 of the NHBRC Home Building Manual</p> <p>Class P site class designation in accordance with 2.5 and 2.8 of Part 1 of Section 2 of the NHBRC Home Building Manual.</p> <p>Class P site class designation in accordance with 2.5 of Part 1 of Section 2 of the NHBRC Home Building Manual.</p>	<p>Masonry houses will require foundation design, building procedures and precautionary measures to be in accordance with Tables 5, 6 and 7 of Part 1 Section 2 of the NHBRC Home Building Manual</p> <p>Houses will require precautionary measures in accordance with NHBRC requirements.</p> <p>Precautionary measures will need to be in accordance with specialist literature.</p>

Sites designated as Class P (contaminated), Class P (controlled fills), Class P (marshy areas), Class P (mine waste fill), Class P (uncontrolled fill), Class P (landfill) and Class P (landslip), Class P (reclaimed areas) and Class P (very soft silts / silty clays) are not generally considered appropriate for development in the subsidy scheme.

Annexure 2: Earthworks classifications for service trenches

Earthworks for service trenches are classified in terms of Tables 2.1 and 2.2.

Table 2.1: Classification of material for machine excavation (SANS 1200 D)

CLASSIFICATION	DESCRIPTION
Restricted excavation	
Soft	Material which can be efficient removed by a back-acting excavator of fly wheel power >0,10 kW for each mm of tined bucket width.
Intermediate	Material which can be removed by a back-acting excavator having a fly wheel power > 0,10kW for each mm of tined-bucket width or with the use of pneumatic tools before removal by a machine capable of removing soft material.
Hard Rock	Material that cannot be removed without blasting or wedging and splitting.
Non-restricted excavation	
Soft	Material which can be efficiently removed or loaded, without prior ripping, by any of the following plant: a bulldozer or a track type front end loader having an approximate mass of 22 tonne and a fly wheel power of 145 kW. a tractor-scraper unit having an approximate mass of 28 tonne and fly wheel power of 245 kW, pushed during loading by a bulldozer equivalent to that described above.
Intermediate	Material which can be efficiently ripped by a bulldozer having an approximate mass of 35 tonne and a fly wheel power of 220 kW.
Hard Rock	Material that cannot be efficiently ripped by a bulldozer having an approximate mass of 35 tonne and a fly wheel power of 220 kW.
Boulder class A	Material containing more than 40% by volume of boulders of size between 0,03 m ³ and 20m ³ , in a matrix of soft material or smaller boulders.
Boulder class B	Material containing 40% or less by volume of boulders of size between 0,03 m ³ and 20m ³ , in a matrix of soft material or smaller boulders.

Table 2.2 Classification for material for hand excavation (Watermeyer, RB. Mobilising the Private Sector to Engage in Labour Based Infrastructure Works: A South African Perspective. Sixth Regional Seminar for Labour-Based Practitioners. Ministry of Works, Transport and Communications in collaboration with ILO/ASIST, Jinja, Uganda, October, 1997.)

Criteria for classifying material as soft excavation Class A *		
Dynamic cone penetrometer - minimum number of blows required to penetrate 100 mm	Granular materials	Cohesive materials
	7-15 +	6 to 8 +
Consistency	Dense - high resistance to penetration by the point of a geological pick; several blows required for removal of material.	Stiff / Very stiff Stiff - can be indented by thumb-nail; slight indentation produced by pushing geological pick point into soil; cannot be moulded by fingers. Very stiff - indented by thumb-nail with difficulty; slight penetration of point produced by blow of geological pick.

* Soft excavation Class A is material which, using a pick or equivalent hand swing tool, can only be excavated with difficulty.

+ Only applicable to materials comprising not more than 10% gravel (Particles having dimensions ; 2.5 mm) of size less than 10 mm and materials containing no isolated small boulders.

Annexure 3: Summary of Buttrick, Van Schalkwyk, Kleywegt and Watermeyer's Method for dolomite land hazard and risk assessment in South Africa

Using the geomorphological, geological, borehole, gravity and geohydrological information, the site shall be zoned in terms of eight Inherent Risk Classes and four Development Risk Categories as described in Tables 3.1 and 3.2.

Inherent Risk Classes are to be determined through the evaluation of the following parameters using the borehole information:

- Receptacle development
- Mobilisation agencies
- Potential development space
- Nature of the blanketing layer
- Bedrock morphology

The procedural guidelines for site characterization are as follows:

Step 1: Undertake preliminary zoning of the site into morphological units using geophysical tools (particularly gravity surveys), remote sensing techniques and field information (geological and topographic mapping). These zones are areas that are anticipated to have similar subsurface geological conditions (rock types, depth to bedrock, extent of weathering, residual materials etc).

Step 2: Boreholes are drilled to investigate the preliminary zones. The stability characterisation procedures are as described / briefly outlined above. The method requires hypothesising the probable impact of a development on the dolomite environment. The potential stability of the land is reviewed in the context of both a dewatering or non-dewatering scenario. Factors to be evaluated are mobilising agencies, receptacle development, potential development space, natural blanketing layer, mobilisation potential of the blanketing layer and bedrock morphology.

Step 3: The individual borehole characterisations and all available information is integrated. The spatial representation of the individual boreholes is decided on. It is essential that the lateral continuity of the conditions represented by individual boreholes is established. Borehole information represents point data and geological conditions vary between boreholes. The use of the geological mapping, gravity survey data and the borehole information will prove invaluable in the process of refining zonal boundaries. Particular subsurface conditions will provide the risk characterisation of a zone. The subsurface conditions represented by the various boreholes are used in conjunction with geophysical, karst and bedrock morphology and geohydrology to determine the boundaries of areas of similar geotechnical characteristics and to develop a 'composite' Inherent Risk characterisation for each particular zone.

Step 4: The finalised risk zonation of the site must be described in terms of the eight Inherent Risk Classes (as outlined in Table 3.1).

Step 5: The appropriate development types are to be selected for each zone and suitable precautionary and remedial measures provided. The particular type of development selected determines the Development Risk, indicating long term safety.

Dolomite Area "D" Designation is to be assigned to each Inherent Risk Class on the site. These "D" designations and related typical precautionary measures are outlined below.

General

1. The site and surrounding area shall be shaped to permit the ready drainage of surface water and to prevent ponding.
Drainage ports should be incorporated in boundary walls particularly at the lowest point of the site, to permit the passage of surface runoff.
2. Natural ponds and water courses located within 10m of any structure shall be rendered impervious.
3. Sanitation systems shall not incorporate soak aways.
4. Backwash and other water from swimming pools, shall be discharged into either the storm water or drainage systems as required by the local authority.
The dolomitic stability over the route of any bulk water bearing service should be evaluated.

Table 3.1: Characterisation: Inherent Risk of doline and a specified-size sinkhole forming.

<i>Inherent Risk Class</i>	<i>Small sinkhole</i>	<i>Medium sinkhole</i>	<i>Large sinkhole</i>	<i>Very large sinkhole</i>	<i>Risk of doline formation</i>	<i>Recommended type of development in order to maintain acceptable Development Risk.</i>
Sinkhole diameter	< 2m	2 – 5m	5 – 15m	> 15m		
Class 1	Low	Low	Low	Low	Low # NDS or DS	Residential, light industrial and commercial development provided that appropriate water precautionary measures are applied. Other factors affecting economic viability such as excavatability, problem soils, etc. must be evaluated.
Class 2	Medium	Low	Low	Low	Medium #NDS	Residential development with remedial water precautionary measures. No site and service schemes. May consider for commercial or light industrial development
Class 3	Medium	Medium	Low	Low	Medium #NDS	Selected residential development with exceptionally stringent precautionary measures and design criteria. No site and service schemes. May consider for commercial or light (dry) industrial development with appropriate precautionary measures.
Class 4	Medium	Medium	Medium	Low	Medium #NDS	Selected residential development with exceptionally stringent precautionary measures and design criteria. No site and service schemes. May utilise for commercial or light (dry) industrial development with appropriate stringent precautionary measures.
Class 5	High	Medium	Low	Low	High #NDS	These areas are usually not recommended for residential development but under certain circumstances selected residential development (including lower-density residential development, multi-storied complexes, etc.), may be considered, commercial and light industrial development. The risk of sinkhole and doline formation is adjudged to be such that precautionary measures, in addition to those pertaining to the prevention of concentrated ingress of water into the ground are required to permit the construction of housing units.
Class 6	High	High	Medium	Low	High #NDS	These areas are usually not recommended for residential development but under certain circumstances highrise structures or gentleman's estates (stands 4 000m ² with 500m ² proven suitable for placing a house) may be considered, commercial or light industrial development. Expensive foundation designs may be necessary. Sealing of surfaces, earth mattresses, water in sleeves or in ducts, etc.
Class 7	High	High	High	Low	High #NDS	No residential development. Special types of commercial or light industrial (dry) development only (eg. bus or trucking depots, coalyards, parking areas). All surfaces sealed. Suitable for parkland.
Class 8	High	High	High	High	Low-High *NDS or DS	No development, nature reserves or parkland.

* = Number of anticipated events per hectare over a period of 20 years with poor design & management

= Non Dewatering Scenario and Dewatering Scenario

Table 3.2: Dolomitic Area Designations

Dolomitic Area Class	Description	Typical Foundation Solutions (Masonry Structures)
D1	No precautionary measures are required to permit the construction of housing units due to an adequate overburden thickness.	Foundations in accordance with the Joint Structural Division's Code of Practice (1995)
D2	The risk of sinkhole and doline formation is adjudged to be such that only general precautionary measures , which are intended to prevent the concentrated ingress of water into the ground, are required to permit the construction of housing units.	Foundations in accordance with Joint Structural Division's Code of Practice (1995).
D3	The risk of sinkhole and doline formation is adjudged to be such that precautionary measures in addition to those pertaining to the prevention of concentrated ingress of water into the ground, are required to permit the construction of housing units.	<p>Possible solutions where sinkholes occur include the provision of:</p> <ul style="list-style-type: none"> reinforced concrete grids incorporating floor spanning between exposed pinnacles (Wagener, 1985) mattresses of improved material (Wagener, 1985) raft foundations to allow occupants to escape and limit damage in the event of a sinkhole occurring (JSD 2000) <p>Possible solutions where dolines are expected include split construction and raft construction in accordance with the Joint Structural Divisions Code of Practice (1995).</p>
D4	The risk of sinkhole and doline formation is such that precautionary measures cannot adequately reduce such risks to acceptable limits so as to permit the construction of housing units or the precautionary measures which are required are impracticable to implement.	-

Township services

5. Underground services shall be designed and constructed so as to minimise maintenance requirements and any potential leakage points in wet services and shall, as far as possible, be designed to avoid possible disturbance of the underground environment.
6. The relevant provision of SANS 1200 DB, L, LB, LC, LD and LE shall be observed in the installation of all underground services.
7. The backfilling to service trenches and other excavations shall, except in rock, not be more permeable than the surrounding material.
8. The stormwater drainage and sewerage system shall incorporate measures to ensure watertightness of conduits and other compartments. Whenever possible, storm water should be channeled in lined, surface canals.
Concrete non-pressure pipes should be of the spigot and socket type with rubber ring seals. Joints in box culverts, channels, etc. should be sealed.
9. Storm water drainage conduits shall be constructed at gradients, which will not permit the deposition of silt, or sand, of the type present in the catchment area.
10. Water mains shall be laid only in road reserves.
11. Water piping materials shall be one or more of the following:
 - pipes of 75mm and larger diameter:
 - high impact PVC pipes with vitaulic joints.
 - steel pipes with internal and external corrosion protection or other flexible (as defined in SANS 10102 Part 1) water pipes with flexible, self anchoring connections.
 - pipes having a diameter of less than 75mm:
 - PE-63 or PE-80 piping in accordance with the provisions of SANS 4427 ISO 4427
 - polypropylene piping

The piping used in mains and communication pipes should be flexible, joints should be minimal in number and, be of the flexible, self anchoring type, i.e. not reliant on thrust blocks or friction for their anchorage.
12. Provision for future connections shall be made in order to minimise the cutting into pipes to provide such connections.
13. Provision shall be made in all water bearing pipelines to accommodate any potential differential movements without causing the pipeline or joints to leak.
14. Road surfaces shall be located sufficiently low so as to permit the drainage of erven onto them.
15. Roadways, which have a gradient of less than 1:80, shall be surfaced/sealed.

16. Where un-surfaced roads are the sole storm water system in a township, the roadways, which act as major storm water collectors, shall be surfaced.
17. The velocity of the 1 in 20 year storm water, flowing along un-surfaced roadways shall not exceed 1,5 m/s.

Plumbing

18. Water pipe entries into the buildings shall be in accordance with Figure 3.1.
19. All sewer and water pipes and fittings shall be provided with flexible, watertight joints.
20. No plumbing and drainage pipes shall be placed under floor slabs, as far as is practicable.
21. The fall of the trenches shall be away from the buildings.
22. Pipes through walls shall be sleeved to permit relative movement.
23. WC pans shall be provided with a flexible connection at the junction with the outlet pipe.
24. The selection of piping material shall take cognisance of corrosion (both external and internal).
25. Water pipes shall have a minimum cover of 500mm.
26. Wherever practical, service trenches shall not be excavated along the length of housing units within the first 3,0m beyond the perimeter of such units.

Site precautions

27. Down pipes, if provided, shall discharge into concrete line drainage channels, which discharge the water at least 1,5m away from buildings.
28. Where guttering is not provided, a 1,5m wide impervious apron slab shall be provided.
29. The ground immediately against the buildings shall be shaped to fall in excess of 75mm over the first 1,5m beyond the perimeter of the building, from where it shall drain freely away from housing units. Apron slabs, where provided shall have the same fall.

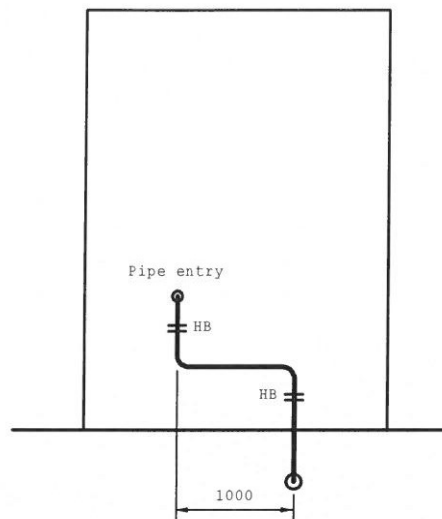
The matters to be considered when establishing and maintaining a Risk Management Plan include:

1 New Townships

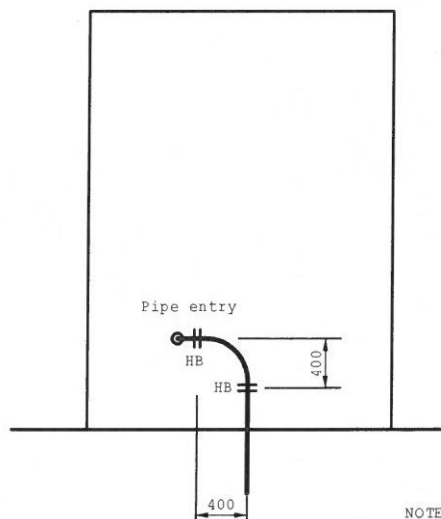
- 1.1 Bulk and internal services in new townships must be installed in accordance with the aforementioned precautionary measures and any additional provisions provided in the geotechnical report.
- 1.2 A register of townships in areas designated as being D1, D2, D3 and D4 (Risk Classes 1 to 8) should be opened. Specified precautionary measures should be entered into the register where they differ from the aforementioned precautionary measures.
- 1.3 The local authority must ensure that bulk services are upgraded appropriately in relation to increasing residential densification.

2. Raising awareness

- 2.1. A map of all known dolomite areas within the local authorities area of jurisdiction should be prepared and maintained. This map should provide a composite stability zonation based on the Dolomite Area Designations and the related Dolomite Risk Class e.g. D2 (Risk Class 1) and D3 (Risk Class 4). The Dolomite Area Designation will be of immediate importance to civil engineers involved in service design and maintenance, whereas the Dolomite Risk Class will be of value to the dolomite risk specialists from a development perspective.
- 2.2 The sections / departments of local authorities responsible for the maintenance of the water, sewer and electrical reticulation and bulk services as well as the building control section should be issued with maps showing the D2, D3 and D4 (Risk Classes 1 to 8) areas and must be informed of the potential risks and maintenance requirements for services in these areas.
- 2.3 Councillors whose wards fall within D1, D2, D3 and D4 (Dolomite Risk Class 1 to 8) areas, as well as leaders of community structures and organisations whose constituents reside in D2 (Classes 1 to 4) and D3 (Classes 3 to 5) areas should be informed of the potential risks and maintenance requirements for services in these areas and the necessity to report any leakage / blockages / ponding of water in these areas to designated Council officials.
- 2.4 Officials who receive and log reports from the public on disruptions in services etc. must be provided with contingency plans including maps showing D2 (Classes 1 to 5), D3 (Classes 3 to 5) and D4 (Classes 6 to 8) areas and must be briefed on the implications of leaks and the like in these areas. Special reporting procedures must be established to ensure that maintenance teams are promptly advised of leaks and the like in areas designated as being D2, D3 and D4.
- 2.5 The local authority should inform residents in areas designated D2 (Classes 1 to 5) and D3 (Classes 3 to 5), every two years in a written communication, of the risks and their responsibilities which will include:
 - * Prompt reporting of leaks and any subsidence.
 - * Refraining from making illegal connections and proceeding with the erection of new buildings on properties and the installation of swimming pools without permission.



RIGID / SEMI FLEXIBLE PIPE



FLEXIBLE PIPING

NOTE:

HB - Holderbats.

Clamp type holderbat not to be fully tightened to allow for movement.



Figure 33.1
Water pipe entry points

- * Ensure that water does not dam up on their properties.

3. Maintenance of services

- 3.1 A pro-active maintenance strategy for water bearing infrastructure should be developed. This can be readily done by superimposing the waterbearing infrastructure on the stability risk zonation map described in Section 2.1 above. Priority in terms of vigilance, general maintenance, repair of leaks and expenditure of funds for upgrading or service replacement can be assigned on the basis of risk exposure. In this manner a prioritised, co-ordinated and pro-active strategy for maintenance and review of waterbearing infrastructure can be developed by the local authority.
- 3.2 Areas designated as being D2, D3 and D4 (Dolomite Risk Classes 1 to 8) must receive priority in the repair of leaks arising from the sewer and water reticulation.
- 3.3 Sewer mains in areas designated as D2, D3 or D4 (Dolomite Risk Classes 1 to 8) should be checked for water tightness by means of an air test at intervals not exceeding two years and repairs undertaken where necessary.
- 3.4 The stormwater systems in areas designated as being D2, D3 or D4 (Risk Classes 1 to 8) should be inspected for blockages and leaks at intervals not exceeding one year and repairs / cleaning undertaken where required.
- 3.5 All bulk services which are located in areas designated as being D2, D3 and D4 should be inspected for water tightness / blockages at intervals not exceeding one year and cleared/repared where required.
- 3.6 Priority should be given to the upgrading of services in areas designated as being D2, D3 and D4 in order to minimise sewer overflows, ponding of water, bursts, water losses, etc.

4. Management of improvements to properties

- 4.1 Building control officers must in areas designated as being D2 and D3, enforce any restriction regarding swimming pools and must ensure that alterations and additions are in accordance with the NHBRC requirements.
- 4.2 Building control officers should once every two years visually inspect properties in areas designated as being D2 and D3 to ensure that water is not damming up on properties.
- 4.3 Building control officers must not permit any densification of properties in areas designated as being D1, D2 or D3 unless it is confirmed by a competent person that such densification does not change the area designation.

5. Measures to prevent land invasion

The local authority must put in place a policy and measures to preclude land invasions and to act positively where such invasions have occurred.

6. Ground water control measures

Artificially induced fluctuations in the dolomite ground water level, particularly where shallow, may trigger sinkhole or doline formation. Consequently, it is essential that local authorities liaise with the Department of Water Affairs and set up appropriate groundwater monitoring procedures. Depending on the Dolomite Risk Class and Dolomite Area Designation (e.g. D4 or Class 7 and 8) of an area, in certain sensitive groundwater compartments, an outright ban on the sinking of abstraction boreholes may be required.

7. Emergency reaction plan in the event of a sinkhole or doline occurring.

The local authority should set in place an emergency reaction plan to be followed in the event of a sinkhole or doline occurring in their area of jurisdiction. Managers of emergency services should be provided with the dolomitic zone designation and risk map and briefed on the implications thereof. It is essential that these managers and emergency services personnel fully understand what a sinkhole is, possible stages of development and how large an area to evacuate around a potential event.

8. Data base of ground subsidence events and structural damage.

The local authority should establish a data base of ground subsidence events and reported structural damage. Detailed records of this nature are useful in developing a clear perspective of the stability situation in a township, highlight areas of weakness and assists in the installation and management of a pro-active maintenance strategy.

NOTE: *The policy should not cause residents to be concerned to live in dolomitic areas. It is perfectly safe to do so provided that certain precautionary measures are observed.*

PART C 4:

SITE INFORMATION

- C 4.1 Scope**
- C 4.2 Subsoil Investigations, Borehole Records
and Test Results**
- C 4.3 Information about Piped and Other Services
Below the Surface of the Site**

**DEPARTMENT OF CO-OPERATIVE GOVERNANCE, HUMAN SETTLEMENTS AND
TRADITIONAL AFFAIRS OF THE NORTHERN CAPE**

TENDER NO. NC/03/2022

**ZF MGCAWU 5 INDIVIDUALS: THE CONSTRUCTION OF 5 BNG HOUSES
THROUGHOUT THE ZF MGCAWU DISTRICT**

C 4: SITE INFORMATION

C 4.1 SCOPE

The documentation included describes the site as at the time of tender to enable the Tenderer to price his tender and to decide upon his method of working and programming.

Work will be executed in a residential area and the Contractor will take **all** necessary steps to ensure the safety of people, animals and/or property.

C 4.2 SUBSOIL INVESTIGATIONS, BOREHOLE RECORDS AND TEST RESULTS

The material on site varies.(But Rock and Hard excavation is expected)

Trail holes were excavated. See C 3.1.3.

C 4.3 INFORMATION ABOUT PIPED AND OTHER SERVICES BELOW THE SURFACE OF THE SITE FOR CONTRACTS INVOLVING GROUND WORKS, AND ABOUT HOOK-UP AND BOUNDARY DETAILS FOR CONTRACTS WITH PLANT INTERFACES, IN ADDITION TO ANYTHING ABOUT THE PHYSICAL SITE WHICH IMPACTS UPON THE CONTRACT

All existing services that could be indicated by the Employer are shown on the Drawings. The Contractor will however investigate on site with the Engineer to identify any existing services that are not indicated on the Drawings before any work commences in an area.

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