DOLOMITE STABILITY INVESTIGATION REPORT: ESKOM ESSELEN SUBSTATION, ESSELEN, TEMBISA, GAUTENG, SOUTH AFRICA



Client: ESKOM



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1. EXECUTIVE SUMMARY

Eskom Esselen Substation has had several occurrences of sinkhole manifestations on the western flank of the site. Efforts have been made to rehabilitate the sinkholes as and when they occur. This approach has seen successful rehabilitation of sinkholes at the site, but due to consistent occurrences of new sinkholes at different localities within the site, Eskom, following the advice from Thoka Geosciences, has decided on conducting a dolomite stability investigation to unveil the extent of the geotechnical dolomite stability within the site to inform the organisation of the problem and expectation for future planning for the Eskom Esselen Substation. Therefore sinkhole rehabilitation was rested whilst a full investigation was initiated at the site.

This report addresses the findings of a dolomite stability investigation requested by Eskom Holdings SOC Limited for the Eskom Esselen Substation located in Esselen Park, Tembisa, Gauteng Province. Thoka Geosciences was appointed by Eskom SOC Limited to conduct a full dolomite stability investigation for Esselen Substation; the request for this investigation is dated 12/05/2017 as per purchase order number 4502641208.

Esselen Substation covers a surface area of about 19 hectare measuring from the outer perimeter fence. This investigation is solely for the area bound by the outer parameter fence and the HV Plant, which covers about 12.5 hectares of the total 19 hectares that makes up Eskom Esselen Substation. The substations servitude extends beyond the outer parameter fence, but did not form part of this investigation as per the instructions from the client. It is however worth noting that some of the rehabilitated sinkholes extended beyond the outer parameter fence including the currently existing sinkhole at the site.

The site is located in Esselen Park, Tembisa bounded by the R21 Highway on the west and the R25 on the south. The central coordinates of the site are Latitude -26.005312° Longitude 28.268662°. The immediate vicinity of Eskom Esselen Substation is occupied by agricultural farms and vacant land. The site is accessible from an unnamed road joining from the M57 road.

On the regional scale, the site is characterised by a slope of 1% sloping towards the eastern direction with a low relief towards the Rietvlei River system located about 3 km east of the site. On a local scale, the site is locally characterised by a generally flat topography gently sloping towards the eastern direction.

The site is currently used as a substation belonging to Eskom. About 5 hectares located at the centre of the substation is used as the HV Plant and the remainder of the periphery is occupied by masonry structures used as offices and parking, open field and transmission networks.

The assessment of the geology within the site boundaries indicated outcropping dolomite at isolated parts of the site interpreted to be panicles of dolomite. Test pits which were excavated for isolation of underground services and the boreholes drilled at the site also indicated occurrence of highly weathered chert breccia in a matrix of ferruginised soil. The typical geological profile at the site is characterised by highly weathered chert breccia in a matrix of reddish brown ferruginsed soil which varies from CLAYEY SILT, SILTY SAND and SANDY GRAVEL. The ferruginsed profile is subsequently typically underlain by highly weathered CHERT which is characterised by occurrence of cavities. The cavities and the highly weathered chert horizon are underlain by moderately; slightly weathered progressing to unweathered DOLOMITE.

The site has had frequent manifestation of sinkholes specifically on the western flank of the site. More than 4 sinkholes have been recorded at the site occurring in a timeframe of 5 years. The sinkholes occur as medium to large sinkholes. Several sinkholes at Esselen Substation were successfully rehabilitated by Thoka Geosciences as a reactionary measure but more sinkholes have manifested at new localities within the site, thus triggering a request to conduct a full dolomite stability investigation at the site. The latest visible sinkhole development at the site may be located by coordinates S26.00678° E28.26650°. It is also worth note that during rehabilitation of the previous sinkholes, Thoka Geosciences discovered some poorly backfilled, re-occurring sinkholes that were supposed to have been rehabilitated by other contractors. As a result, some of these identified sinkholes which were within the working boundary of Thoka Geosciences were properly rehabilitated during the tenure of Thoka Geosciences at the time. A walkover survey outside the outer parameter fence has revealed that there is a number of existing sinkholes which are within the Eskom, Esselen Substation servitude, with some occurring under the Eskom Power lines. Again some of these sinkholes were backfilled with rubble by nearby farmers as an attempt to close off the sinkholes.

Based on the available data, the karst instability in the area is expected to be high considering the number of karst instability events that have already been indicated to be in access of 10 events within vicinity of the site area.

The assessment of the A2N0631 monitoring borehole shows fluctuation of the groundwater level with peak groundwater levels recorded during the year 1997 (7 m from surface) and 2015 (8.7 m from surface). The groundwater levels recorded between 1997 and 2015 shows a drop in the groundwater levels from peak of 7 m from surface to 18 m from surface during the year 2010. The groundwater level begins to drop again from 2015 to the last reading date recording 16.6 m.

Monitoring borehole A2N0627 indicates a steady groundwater level dating back from the year 1999 to date whereby the groundwater remains at about 30 m from surface with fractional fluctuations.

Groundwater borehole A2N0633 only has records up to the year 1998. Recent groundwater data is not available. Records of the groundwater level from 1988 to 1995 show a steady state until a sudden rise in groundwater level between 1995 and 1998 to a depth of about 10 m from surface.

The assessment of the regional groundwater situation within the Sterkfontein Dolomite Compartment shows variable maintenance of the groundwater within the compartment whereby A2N0627 shows sustainable maintenance and A2N0631 shows variable unsteady maintenance of the groundwater level with fluctuations greater than 10 m difference in a 10 year period.

A gravity survey, in support of a dolomite-stability study, was conducted over roughly 12.5 ha surrounding Eskom's Esselen Substation, across the R21 from Tembisa, on 10 October 2017. The area was covered with a 15m grid comprising 622 stations.

The broad gravity high which spans most of the surveyed area is shown to consist of shallow (3 m to 4 m) dolomite bedrock head as intersected by ESN BH20 and ESN BH6. Subtle changes in the residual gravity, within the broader high, correspond with variations in weathering as noted by ESN BH9, ESN BH11, ESN BH13 & ESN BH18 - 19 which intersected competent bedrock formation at between 8 m and 11 m deep. Bedrock deepens toward the northeast, coincident with a broader gravity low, where ESN BH7 - 8, ESN BH15 & 21 intersected bedrock head between 13 m and 18 m deep. In most cases small cavities were intersected which caused sample loss. ESN BH16 & 17, drilled on the gravity gradient, intersected fairly large cavities and were terminated at 19 m and 14 m respectively; both of these locations fall on subtle and small lows originating from the larger gravity low. The gravity low toward the southwest is more irregular and appears to be comprised of two lows, located in the northwest and southwest, connected by narrow, rectilinear lows. This is supported by ESN BH1 & 3 which intersected bedrock at 12 m and 18 m respectively while ESN BH2 & 4 were terminated after intersecting fairly large voids. Although ESN BH5 & 12 showed bedrock to be around 5 m depth, the gravity suggests they may have been drilled on the edge of a slot. Overall the drilling results show the gravity to be a fair reflection of bedrock topography.

A set of two traverse lines were prepared at Eskom Esselen Substation to conduct resistivity surveys using the ABEM LS transmitter plus receiver unit with 61 active field electrodes. The surveys were conducted with minimum electrode spacing of 3 m using the schlumberger and the wenner protocols / electrode array. A maximum distance of 240 m was covered with the setup

and a maximum depth of 52 m was surveyed. The data was assessed and modeled with the RES-2D-INV software.

Traverse line 1 crossing the northeast-southwest direction indicates close to homogenous band of highly resistive material encountered from surface to about 14 m depth. The resistive band also shows pockets of material with even greater resistivity (purple color code). Traverse line 1 passes through vicinity of drilled ESN BH 13, ESN BH 5 and ESN BH 12 which intercept solid dolomite between 4 - 9 m depth inferable as shallow dolomite bedrock.

Traverse line 2 which crosses along a southeast-northwest traverse line for a 240 m distance displays a highly variable geophysical signature which is characterized by isolated pockets of low (dark blue color code) – intermediate (green) resistivity material close to surface. The Traverse line 2 passes within region of the recorded gravity low region northeast of the Esselen Substation characterized by deep dolomite bedrock. On the northern flank of Traverse line 2 from about 180 m to 240 m traverse distance, a resistive material is recorded which coincides the boundary of gravity high zones recorded on the gravity survey.

A total of 19 boreholes were drilled at the site. The boreholes were drilled with a rotary percussion drilling rig (Zeus 5000 rig) at 21 bar air pressure with a capacity of 0.519 m³ per second using the Reverse Circulation Method. A 208 mm clay cutter was utilized instead of a drilling bit on portions of the area underlain by clay material and a 145 mm reverse circulation drill bit was used on rock material.

Borehole logging results indicated occurrence of two groups of geological profiles, one group which is defined by the gravity high zones and higher apparent resistivity signature and the other group defined by gravity low zones and low resistivity signature. The gravity high zones are characterised by shallow dolomite bedrock conditions which vary based on weathered zones of chert and iron oxides/ferruginous soils and cavities. The gravity low zones are characterised by deeper dolomite bedrock overlain by small persistent cavities and soft clay, clayey silt overburden in upwards succession.

Ferruginous Soils

Occurrences of ferruginous soils in the form of SILTY SAND, CLAYEY SILT and ferricrete nodules are encountered at the site to maximum depths of about 7 m from surface. The ferruginous soils are commonly associated with highly weathered CHERT fragments e.g. ESN BH 1, 3, 4, 5, 12, 13, 16, 18 and 21. The occurrence of ferruginous soil is encountered within the gravity high zones of the site associated with shallow bedrock geology.

Transported Soft Clay, Silty Sand,

Soft clay, silty clay material is encountered within the gravity low zones which are associated with deeper dolomite bedrock. The soft clay, silty clay material is typically reddish brown/dark maroon in colour commonly characterised by rapid penetration rates. The silty clay material is in many cases underlain by persistent cavities until solid dolomite bedrock at depth e.g. ESN BH 15 and ESN BH 8.

Weathered Chert of the Malmani Subgroup

Profiles of moderately to highly weathered CHERT occur commonly within the gravity high zones at the site. The highly weathered CHERT is characterised by rapid penetration rates, and subsequent cavities sometimes in excess of 10 meters e.g. ESN BH 2 and ESN BH 4.

Malmani Subgroup Dolomite

Solid dolomite bedrock is encountered as bluish grey sub-angular chip samples. The solid dolomite bedrock occurs either as shallow bedrock associated with the gravity high zones whereby solid dolomite sometimes rests at 4 meters from the surface. The deep dolomite bedrock is encountered on gravity low zones whereby solid dolomite bedrock can be encountered deeper than 18 m from surface e.g. ESN BH 2 & 3.

The assessment of regional groundwater indicates groundwater levels to be at maximum depth of about 30 m from the surface for monitoring station A2N0627.

None of the 19 investigation boreholes at site encountered groundwater. The deepest borehole drilled at site reached depth of 24 m from the surface. This infers a deeper groundwater table at the site and that sinkhole or subsidence associated with groundwater drawdown is unlikely but rather ingress from ponding / percolating surface water is the main trigger for sinkhole formation at the site.

Two additional deeper boreholes were drilled on the northeast portion of the site represented by gravity low conditions in preparation for the dolomite risk management strategy requirements. The boreholes were drilled in access of 30 m from the surface and groundwater was intercepted. But, due to instantaneous collapsing during the drilling process, the boreholes had to be prematurely abandoned. The boreholes proved to be too risky to drill with conventional percussion methods, Thoka Geosciences recommends ODEX drilling systems to be implemented specifically for securing groundwater monitoring points at the site.

The purpose of this dolomite stability investigation report is to determine the different engineering geological properties of the surface, subsurface soils and dolomite rocks in

accordance with the SANS 1936 guidelines and assign site classification for the dolomite underlain area.

As required by SANS 1936, stability evaluation of the site included the examination of receptacle development potential, mobilising agents, mobilization potential of blanket layer, bedrock morphology and potential sinkhole development space.

Mobilization is defined as the movement of dolomitic overburden by subsurface erosion. Mobilising agents include ingress water, ground vibrations, water level drawdown or any process that can induce mobilisation of material within the blanketing layer under the force of gravity (Buttrick *et al*, 1995).

The site will be susceptible to water ingress from current and future installation of wet services and water ingress from water ponding or percolation from surface runoff.

As discussed above, only two boreholes intercepted groundwater at depths greater than 24 m from surface, the remainder 19 of the boreholes drilled did not encounter groundwater, thus groundwater is believed to be below the cavity prone dolomite horizons at the site (i.e. >24 m from surface). Thus drawdown of existing groundwater level would have non to little impact on the stability of the dolomitic land as it is not within highly leached / weathered CHERT and DOLOMITE horizons characterised by cavities. This assertion is also reinforced by the fact that groundwater monitoring was conducted within the January – February 2018 period which is expected to be the high rainfall season for the geographical area. Although this is the case, there should be a groundwater management system in place as a precautionary measure to manage the usage/fluctuation of the groundwater particularly focusing on possible rise and fall of the groundwater table that could increase the inherent hazard for ground movement.

According to SANS 1936 - 2, the positive contribution towards dolomite stability of shallow groundwater levels should only be taken into account where:

- The compartmentalization of dolomite groundwater and aerial extent of the compartments have been determined.
- All groundwater users are identified and the maximum quantities abstracted are known.
- There is evidence to show licensed pumping does not exceed normal natural replenishment.
- There are data showing the groundwater level behaviour patterns during periods of drought and recovery.
- The groundwater level is not located within the dolomite residuum.
- The local authority or relevant national department has regularly monitored the groundwater level in the past and continues to do so.

More groundwater monitoring stations must be developed to ascertain the above requirements for groundwater. Thus a groundwater monitoring station at the site is recommended as part of a risk management facility.

The method adopted in sinkhole size determination followed the criteria by Buttrick *et al* 2001, whereby bedrock depth shallower than 3 m is expected to be more prone to develop a small (<2 m) sinkhole and bedrock depth >3 m<15 m is more prone to develop a medium sinkhole and bedrock depth deeper 15 m more prone to develop a large sinkhole (5 - 15 m Wide Sinkholes).

Due to the variable depth to dolomite bedrock, the site is inferred to be more prone to develop medium to large sinkholes respective to the borehole results. Medium sinkholes are sinkholes expected to have a nominal diameter of 2-5 m and large sinkhole expected to have 5-15 m diameter. Based on the assessment of the geophysics, gravity low zones have deeper bedrock and more prone to develop large sinkholes and gravity high zones are more prone to develop medium to large sinkholes.

The blanketing material at the site may be summed up as ferrugenised SILTY SAND, SILT and SOFT CLAY with fragments of highly weathered CHERT. The geological profiles of the blanketing material at the site although sometimes as thick as 8 m from surface is easily permeable due to the nature and characteristics of the material. Drilling penetration rates recorded to be less 1 minute per meter drilled over the blanketing material suggest easily permeable material susceptible to collapse/movements. Thus instability triggered by water ingress from the surface is highly likely in situations whereby surface water is not drained at sufficient rate.

The occurrence of highly weathered chert/dolomite horizons and cavity at shallow and deeper depths as per gravity high and gravity low zones throughout the site suggests typical high inherent hazard for karst formation as voids and cavities form part of receptacles for material of the overlaying strata. Recorded historic sinkhole and subsidence on the western flank of the site further imply high risk zones and caution. The vast area on the western flank of the Eskom Esselen Substation has been zoned of as a high risk area as a whole due to evident sinkhole formation within that area. The triggering mechanism for sinkhole formation is by assessment due to sudden loss of support accelerated by surface water ingress.

Considering all the discussed attributes of mobilization /blanketing material agency, cavities, sinkhole size and nature of blanketing material, dolomite bedrock topography and geohydrology, the site is regarded to have variable hazard classes. Boreholes representing high inherent hazard (IHC 6-8) dominate by about 70 % of the drilled borehole classifying either as IHC

6/7/8. Most of the high inherent hazard areas are characterised by either gravity low zones/deep bedrock or contact zones between gravity high and gravity low zones e.g. ESN BH 13. This assessment suggests platforms of shallow dolomite adjacent to troughs of weathered zones covered by soil sediment deposits (FERRUGENISED CLAYEY SILT). The platforms of shallow dolomite in most instances represent intermediate inherent hazard classified as IHC 2/3/4.

The Dolomite Land Classification for the site will be pre-determined by the type of development to be pursued at the site. SANS 1936-1, Table 2 shows the permissible land usage per inherent hazard class for dolomite land. Any proposed development will thus be guided by the permissible land usage guidelines. The site falls under C3 type development, thus developable with footprint requirements up until IHC 6. Therefore any future development at the site should be prioritised on the gravity high zones of the site but with care to avoid the contact zones of gravity high platforms and gravity low troughs. No future development at the site should be conducted or permitted without a detailed footprint investigation.

Portions of the site are thus regarded to be conditionally developable under D3 + Footprint Investigation conditions as a C3 Development Type. Should any future footprint investigations proof worse conditions, the respective area must be regarded as a D4 area with high inherent hazard.

A crucial requirement of development on dolomitic lands, as stipulated in SANS 1936-1, is the establishment and implementation of a dolomite risk management strategy (DRMS) by the developer, which must tie in with that in place and controlled by the relevant local authority which in this regard the relevant authority would be the Ekurhuleni Municipality. SANS 1936-4 describes this in detail.

It is therefore highly recommended that Thoka Geosciences be appointed to develop a dolomite risk management strategy so as to minimize further development of sinkholes within the station. The dolomite risk management strategy will entail amongst others, monitoring regional groundwater, monitoring local groundwater fluctuations, monitoring of surface flow on site and emergency preparedness plan in cases where sinkholes develop.

The risk management strategy will be tied with local municipalities risk management strategy in place also providing reporting strategies of sinkhole and subsidence events. The training of local personnel occupying the Esselen Substation will be part and parcel of the risk management strategy. Training will include amongst other aspects, training of personnel on identification of sinkholes, subsidence, tension cracks leading to sinkholes, and management of surface water ponding and reporting strategies.

Urgency to rehabilitate existing sinkholes must be placed and further assessment of the dolomite stability beyond the outer parameter fence onto the boundary of Eskom Esselen Substation servitude needs to be prioritised as a matter of urgency.

Areas deduced to be IHC 8 from this investigation are recommended for proactive rehabilitation in the form of overburden strip, expose cavity throat and rehabilitation to prevent highly likely sinkhole occurrences.

2. INTRODUCTION

2.1. BACKGROUND

Eskom Esselen Substation has had several occurrences of sinkhole manifestations on the western flank of the site. Efforts have been made to rehabilitate the sinkholes as and when they occur. This approach has seen successful rehabilitation of sinkholes at the site, but due to consistent occurrences of new sinkholes at different localities within the site, Eskom, following the advice from Thoka Geosciences, has decided on conducting a dolomite stability investigation to unveil the extent of the geotechnical dolomite stability within the site to inform the organisation of the problem and expectation for future planning for the Eskom Esselen Substation. Therefore sinkhole rehabilitation was rested whilst a full investigation was initiated at the site.

2.2. GENERAL

This report addresses the findings of a dolomite stability investigation requested by Eskom Holdings SOC Limited for the Eskom Esselen Substation located in Esselen Park, Tembisa, Gauteng Province. Thoka Geosciences was appointed by Eskom SOC Limited to conduct a full dolomite stability investigation for Esselen Substation; the request for this investigation is dated 12/05/2017 as per purchase order number 4502641208.

2.3. OBJECTIVES

The objectives of the investigation were to:

- Assess the exposure and vulnerability of the site to dolomite geo-hazards which have historically manifest into sinkholes and surface subsidence.
- Characterize the suitability of the site for any future development
- Highlight any discrepancies noted on the site in aspects of geotechnical stability risk
- By default, the following form part of the investigations:
- Determination of the site specific geological conditions
- Determination of all potentially restraining dolomite stability factors including the hydrogeological characteristics and soils
- Establish the presence and proximity of groundwater to the surface including historic groundwater data
- Classification of the site according to SANS 1936 in respect of dolomitic land
- Provide recommendations for future utilization of the site, risk management solutions

2.4. SCOPE

The scope of the work conducted to achieve the objectives of the investigations involved:

- A site visit which involved touring the vicinity of the site boundaries and capturing of information such as rock outcrops, geology, sinkholes, subsidence, tension cracks, installed services, vegetation, soils, topographic properties and access routes.
- Desktop investigation of all available information and previous work in the vicinity of the area
- Preparation of a gravity survey of the site
- Preparation of borehole positions integrated with gravity survey results
- Preparation for rotary percussion borehole drilling using reverse circulation methods
- Drilling of 19 boreholes at selected positions within the site using the most efficient methods
- Capturing of drilling information including groundwater strikes and groundwater levels after 24 period lapse
- Interpretation of information gathered from all methodologies employed
- Recommending precautionary and risk mitigation measures to be implemented
- reparation of a comprehensive report

2.5. Sources of Information

- The 1:250 000 2628 East Rand Sheet as published by the Council for Geoscience
- Council for Geoscience Sinkhole Database, Borehole and Dolomite Stability Investigation Reports
- South Africa National Standards (SANS-1:2012, -2:2012 and -3:2012)
- Satellite imagery available from Google Earth
- National Groundwater Achieves
- Published online literature relating to the site geology, geo-hydrology and physiographic information of the site

3. SITE LOCATION AND DESCRIPTION

3.1. GEOGRAPHIC DESCRIPTION

Esselen Substation covers a surface area of about 19 hectare measuring from the outer perimeter fence. This investigation is solely for the area bound by the outer parameter fence and the HV Plant, which covers about 12.5 hectares of the total 19 hectares that makes up Eskom Esselen Substation. The substations servitude extends beyond the outer parameter fence, but did not form part of this investigation as per the instructions from the client. It is however worth noting that some of the rehabilitated sinkholes extended beyond the outer parameter fence including the currently existing sinkhole at the site.

The site is located in Esselen Park, Tembisa bounded by the R21 Highway on the west and the R25 on the south. The central coordinates of the site are Latitude -26.005312° Longitude 28.268662°. The immediate vicinity of Eskom Esselen Substation is occupied by agricultural farms and vacant land. The site is accessible from an unnamed road joining from the M57 road.

3.2. TOPOGRAPHY

On the regional scale, the site is characterised by a slope of 1% sloping towards the eastern direction with a low relief towards the Rietvlei River system located about 3 km east of the site. On a local scale, the site is locally characterised by a generally flat topography gently sloping towards the eastern direction.

Figure 1 shows a delineation of the topography and the slope of the site.

3.3. CLIMATE

The climate data is based on a 30 year historical weather data for Tembisa, Gauteng. The mean low temperatures range between 3 to 15 degree Celsius and the mean high temperatures range between 18 to 28 degrees Celsius. The rainfall in the area is seasonal with the highest rainfall occurring during the October to March period ranging from about 71 mm to about 127 mm. The high rainfall period is within the summer months (https://www.meteoblue.com/en/weather/forecast/modelclimate/tembisa_south-africa_949880, March 2017).

3.4. VEGETATION

The site vegetation is characterised by the Carletonville Dolomite Grassland which forms part of the Dry Highveld Grassland Bioregion.

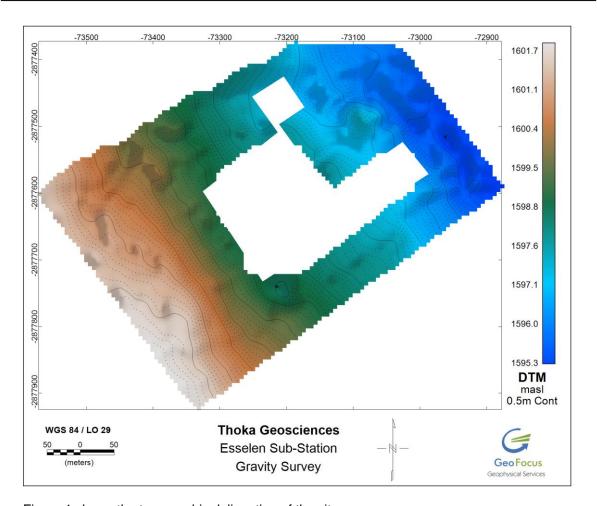


Figure 1 shows the topographic delineation of the site

3.5. PREVIOUS AND CURRENT LAND USE

The site is currently used as a substation belonging to Eskom. About 5 hectares located at the centre of the substation is used as the HV Plant and the remainder of the periphery is occupied by masonry structures used as offices and parking, open field and transmission networks.

3.6. DRAINAGE

The natural drainage within vicinity of the site is towards the east into the Rietvlei River System. The drainage is controlled by a gentle slope of about 1%. The locality of the site is engineered with concrete surface V-channels drainage systems that induce control and channelling of storm water towards the eastern flank of the site (Figure 2).



Figure 2 shows engineered storm water drainage systems developed at the site.

3.7. REGIONAL GEOLOGY

The regional geology of the area is underlain by the Malmani Subgroup dolomitic rocks of the Chuniespoort Group (1: 250 000 Geological Series, 2628 East Rand). The Malmani Subgroup is comprised of the Oaktree Formation as the oldest formation followed by the Monte Christo, Lyttelton, Eccles and Frisco Formation in chronological order from oldest to youngest of age. The subdivision of the Malmani Subgroup is based on the differences in chert content, stromatolite morphology, intercalated shales and erionsion surfaces (Button, 1973b). Due to the composition of dolomite rock, slightly acid fluids such as rain water can cause dissolution of dolomite. The dissolution process of dolomite can be represented as follows:

 $CaMg(CO3)2 + 2 H2CO3 \rightarrow Ca(HCO3)2 + Mg(HCO3)2$ Equation 1

Due to the solubility of dolomite, vertical zonation of residual products may be of result. The vertical zonation is typically in the form of solid unweathered dolomite bedrock overlain by slightly weathered and jointed bedrock and thereafter a sudden highly weathered dolomite horizon characterised by low strength insoluble manganese oxides, chert an iron oxides. The Council for Geosciences has a dedicated database for sinkholes recorded and all dolomite stability investigations within the regional area. An abundance of dolomite stability investigations is evident within the regional geographical area. Figure 3 illustrates existing boreholes drilled as part of dolomite stability investigations for other properties and occurrences of sinkholes within the area. It is evident that the sinkholes that have occurred within the Esselen Substation have not been recorded under the Council for Geosciences database. This is a concern that must be addressed by a dolomite risk management strategy for Eskom Esselen Substation.

3.8. LOCAL GEO LOGY

The assessment of the geology within the site boundaries indicated outcropping dolomite at isolated parts of the site interpreted to be panicles of dolomite. Test pits which were excavated for isolation of underground services and the boreholes drilled at the site also indicated occurrence of highly weathered chert breccia in a matrix of ferruginised soil. The typical geological profile at the site is characterised by highly weathered chert breccia in a matrix of reddish brown ferruginsed soil which varies from CLAYEY SILT, SILTY SAND and SANDY GRAVEL. The ferruginsed profile is subsequently typically underlain by highly weathered CHERT which is characterised by occurrence of cavities. The cavities and the highly weathered chert horizon are underlain by moderately; slightly weathered progressing to unweathered DOLOMITE.

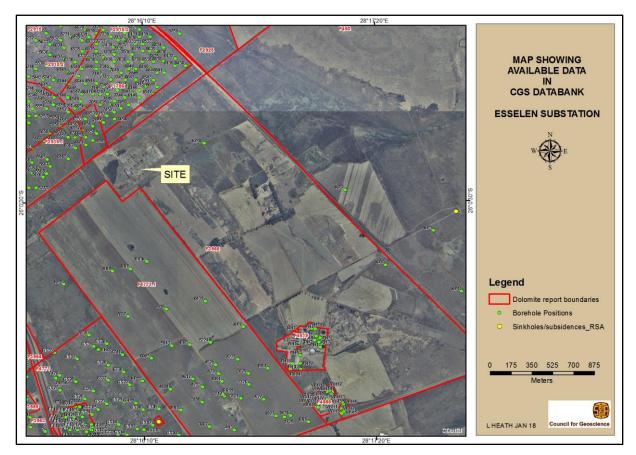


Figure 3: Shows the locality of previous dolomite stability investigation reports and previously drilled boreholes within the vicinity of the investigation area.

3.9. HISTORIC KARST INSTABILITY

The site has had frequent manifestation of sinkholes specifically on the western flank of the site. More than 4 sinkholes have been recorded at the site occurring in a timeframe of 5 years. The sinkholes occur as medium to large sinkholes. Several sinkholes at Esselen Substation were successfully rehabilitated by Thoka Geosciences as a reactionary measure but more sinkholes have manifested at new localities within the site, thus triggering a request to conduct a full dolomite stability investigation at the site. The latest visible sinkhole development at the site may be located by coordinates \$26.00678° E28.26650° (

Figure 4). It is also worth note that during rehabilitation of the previous sinkholes, Thoka Geosciences discovered some poorly backfilled, re-occurring sinkholes that were supposed to have been rehabilitated by other contractors. As a result, some of these identified sinkholes which were within the working boundary of Thoka Geosciences, were properly rehabilitated during the tenure of Thoka Geosciences at the time. A walkover survey outside the outer parameter fence has revealed that there is a number of existing sinkholes which are within the Eskom, Esselen Substation servitude, with some occurring under the Eskom Power lines. Again some of these sinkholes were backfilled with rubble by nearby farmers as an attempt to close off the sinkholes.

Based on Table 3-Inherent hazard categories, SANS 1936-2; the inherent hazard must also be determined by considering the amount of anticipated events per hectare per 20 years. High Inherent Hazard is implicated by karst associated events occurring in an area of 1 hectare in less than 20 years. Based on the available data, the karst instability in the area is expected to be high considering the number of karst instability events that have already been indicated to be in access of 10 events within vicinity of the site area.



Figure 4 shows the latest sinkhole development at the site

3.10. REGIONAL HYDROGEOLOGY

The site is located within the Sterkfontein Dolomite Compartment in the A21A Groundwater Management Unit in the Crocodile West and Marico Water Management Area. The hydrogeological properties of the compartment are characterised by the karst nature of dolomite aquifers.

Three of the nearest available groundwater monitoring points within a 3 km radius from the Esselen Substation have been selected for assessment of the regional groundwater situation. The groundwater monitoring boreholes are available from the National Groundwater Achieve identifiable as A2N0633, A2N0627 and A2N0631 as depicted by Figure 5, Figure 6 and Figure 7.

The assessment of the A2N0631 monitoring borehole shows fluctuation of the groundwater level with peak groundwater levels recorded during the year 1997 (7 m from surface) and 2015 (8.7 m from surface). The groundwater levels recorded between 1997 and 2015 shows a drop in the groundwater levels from peak of 7 m from surface to 18 m from surface during the year 2010. The groundwater level begins to drop again from 2015 to the last reading date recording 16.6 m.

Monitoring borehole A2N0627 shown in Figure 6 indicates a steady groundwater level dating back from the year 1999 to date whereby the groundwater remains at about 30 m from surface with fractional fluctuations.

Groundwater borehole A2N0633 only has records up to the year 1998. Recent groundwater data is not available. Records of the groundwater level from 1988 to 1995 show a steady state until a sudden rise in groundwater level between 1995 and 1998 to a depth of about 10 m from surface (Figure 7).

The assessment of the regional groundwater situation within the Sterkfontein Dolomite Compartment shows variable maintenance of the groundwater within the compartment whereby A2N0627 shows sustainable maintenance and A2N0631 shows variable unsteady maintenance of the groundwater level with fluctuations greater than 10 m difference in a 10 year period.

Ground stability is an important consideration in establishing large scale water abstraction in dolomite underlain areas. The extent of water level drawdown is one of the critical factors in the development of ground subsidence and sinkhole formation. The risk of development of ground subsidence and sinkholes is greatest in areas where the groundwater level occurs closer to surface (<30 m) and where the groundwater fluctuates more than 6 m in response to pumping (HC Barnard, 2000).

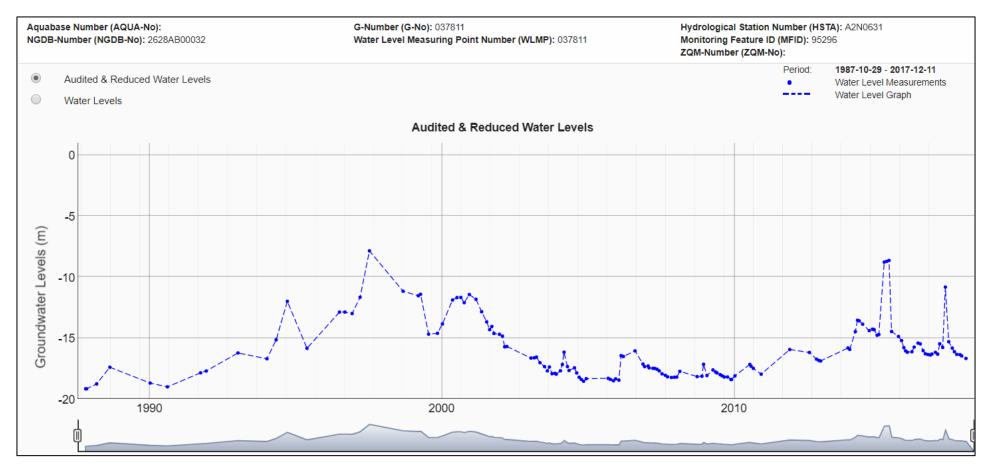


Figure 5 shows the groundwater monitoring graph for monitoring station A2N0631 from the year 1987 – 2017 (National Groundwater Achieve, 2018)

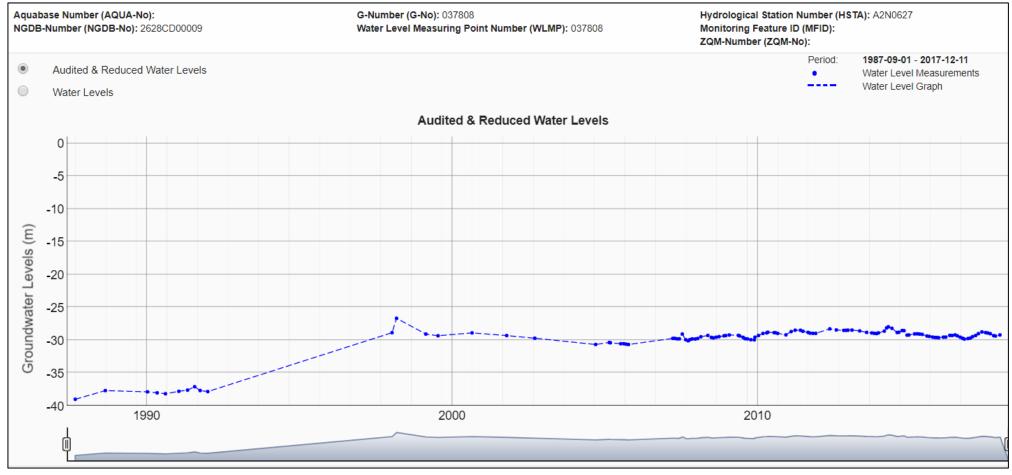


Figure 6 shows the groundwater monitoring graph for monitoring station A2N0627 from the year 1987 – 2017 (National Groundwater Achieve, 2018)

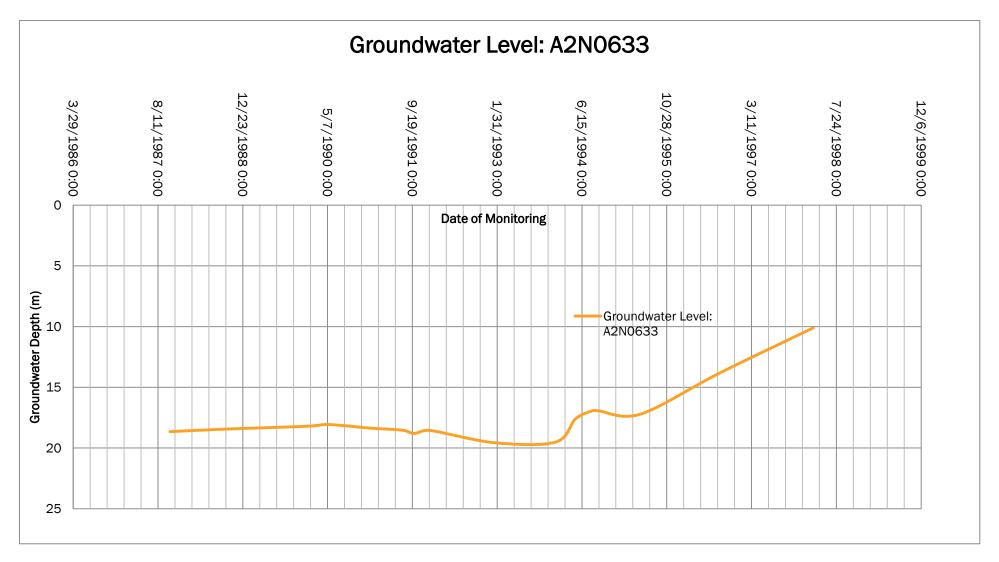


Figure 7 shows the groundwater monitoring graph for monitoring station A2N0633 from the year 1986 – 1999 (National Groundwater Achieve, 2018)

4. GEOPHYSICAL SURVEY RESULTS

4.1. GRAVITY SURVEY

A gravity survey, in support of a dolomite-stability study, was conducted over roughly 12.5 ha surrounding Eskom's Esselen Substation, across the R21 from Tembisa, on 10 October 2017. The area was covered with a 15m grid comprising 622 stations.

Gravity was observed with a Scintrex CG5 gravimeter whilst a Javad DGPS recorded station locations. Gravity data processing procedures commonly used for dolomite studies were applied, firstly reducing the data to relative Bouguer values by applying elevation, terrain and Bouguer corrections. Secondly, an estimation of the regional gravity field (slope), calculated either through linear regression or a grid trend, is removed from the relative Bouguer gravity to produce an uncorrected or provisional residual gravity map emphasizing local changes. The final residual gravity map is produced by correcting for bedrock head intersections acquired during outcrop mapping, pitting and/or drilling. This usually entails a base shift (adjustment by a constant) so that the residual gravity values agree on average with bedrock depths. This is done by shifting the residual Bouguer highs to represent outcrop, where present, as zero (0) mGal. Otherwise drill locations where bedrock was intersected are adjusted so that the corresponding residual Bouguer value approximates the ratio of -1mGal = 48m. In some cases, notably small surveys, bedrock head intersections can provide the necessary spatial correlation needed to recalculate the regional gravity field in order to achieve a better fit.

The median elevation of the survey area is approximately 1,599 masl (relative) sloping southwest to northeast by roughly 7 m. A 1st-order approximation of the regional gravity field was removed from the Bouguer anomaly data resulting in a residual gravity dataset with peak-to-peak distribution of approximately 0.4 mGal. Corrections were applied to the residual gravity data based on a set of nineteen (19) boreholes drilled across the site.

The broad gravity high which spans most of the surveyed area is shown to consist of shallow (3 m to 4 m) dolomite bedrock head as intersected by ESN BH20 and ESN BH6. Subtle changes in the residual gravity, within the broader high, correspond with variations in weathering as noted by ESN BH9, ESN BH11, ESN BH13 & ESN BH18 - 19 which intersected competent bedrock formation at between 8 m and 11 m deep. Bedrock deepens toward the northeast, coincident with a broader gravity low, where ESN BH7 - 8, ESN BH15 & 21 intersected bedrock head between 13 m and 18 m deep. In most cases small cavities were intersected which caused sample loss. ESN BH16 & 17, drilled on the gravity gradient, intersected fairly large cavities and were terminated at 19 m and 14 m respectively; both of these locations fall on subtle and small lows originating from the larger gravity low. The gravity low toward the southwest is more irregular and

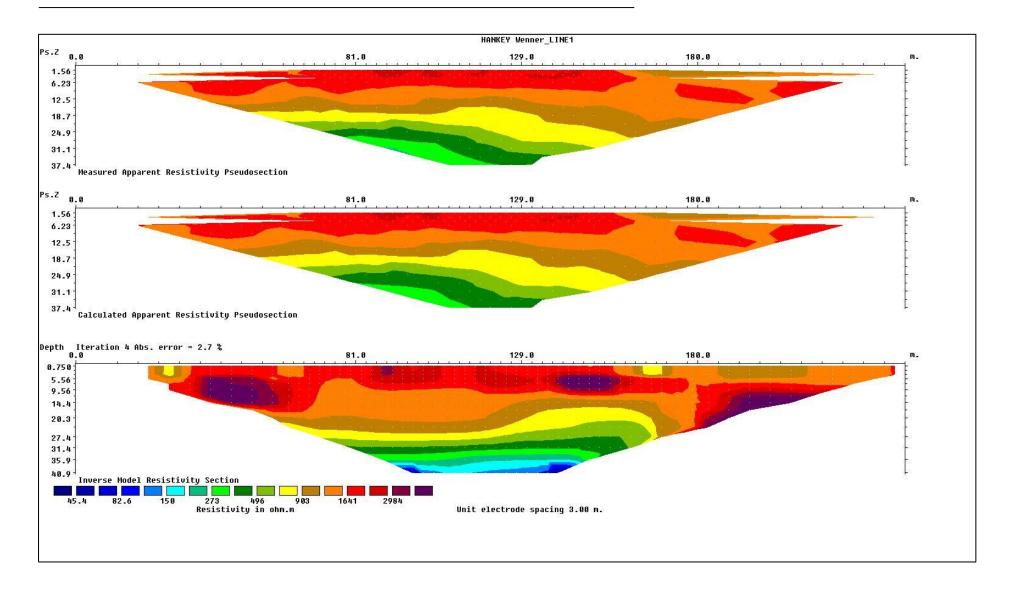
appears to be comprised of two lows, located in the northwest and southwest, connected by narrow, rectilinear lows. This is supported by ESN BH1 & 3 which intersected bedrock at 12 m and 18 m respectively while ESN BH2 & 4 were terminated after intersecting fairly large voids. Although ESN BH5 & 12 showed bedrock to be around 5 m depth, the gravity suggests they may have been drilled on the edge of a slot. Overall the drilling results show the gravity to be a fair reflection of bedrock topography.

4.2. RESISTIVITY SURVEY

A set of two traverse lines were prepared at Eskom Esselen Substation to conduct resistivity surveys using the ABEM LS transmitter plus receiver unit with 61 active field electrodes. The surveys were conducted with minimum electrode spacing of 3 m using the schlumberger and the wenner protocols / electrode array. A maximum distance of 240 m was covered with the setup and a maximum depth of 52 m was surveyed. The data was assessed and modeled with the RES-2D-INV software.

Traverse line 1 crossing the northeast-southwest direction (Figure 8) indicates close to homogenous band of highly resistive material encountered from surface to about 14 m depth. The resistive band also shows pockets of material with even greater resistivity (purple color code). Traverse line 1 passes through vicinity of drilled ESN BH 13, ESN BH 5 and ESN BH 12 which intercept solid dolomite between 4-9 m depth inferable as shallow dolomite bedrock.

Traverse line 2 which crosses along a southeast-northwest traverse line for a 240 m distance (Figure 9) displays a highly variable geophysical signature which is characterized by isolated pockets of low (dark blue color code) – intermediate (green) resistivity material close to surface. The Traverse line 2 passes within region of the recorded gravity low region northeast of the Esselen Substation characterized by deep dolomite bedrock. On the northern flank of Traverse line 2 from about 180 m to 240 m traverse distance, a resistive material is recorded which coincides the boundary of gravity high zones recorded on the gravity survey.



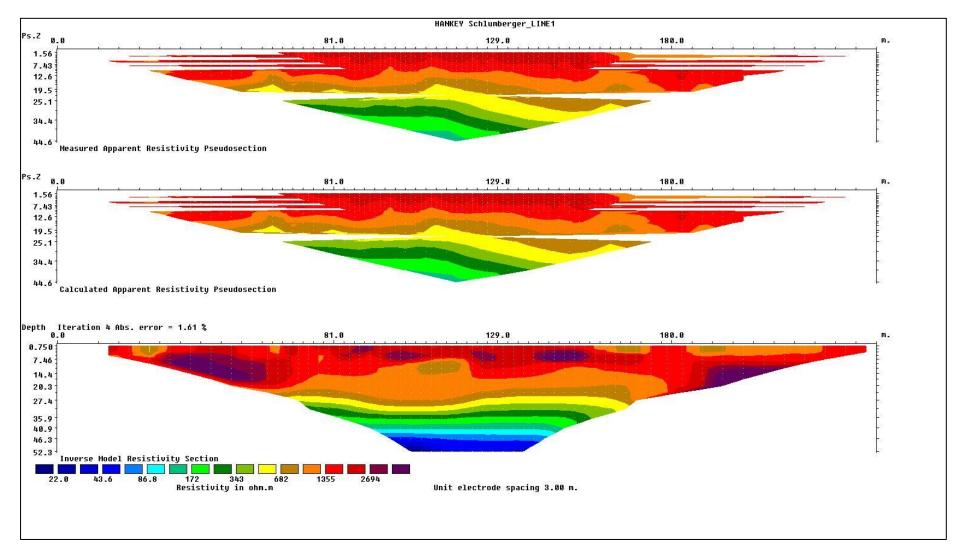
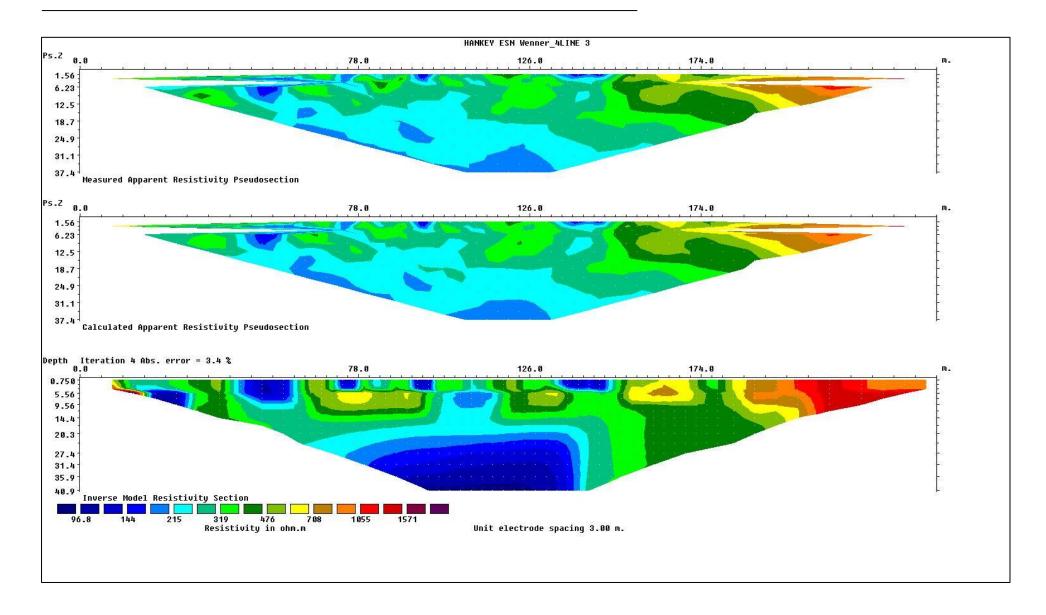


Figure 8 shows the resistivity survey line 1 conducted along a northeast-southwest direction over a 240 m traverse line



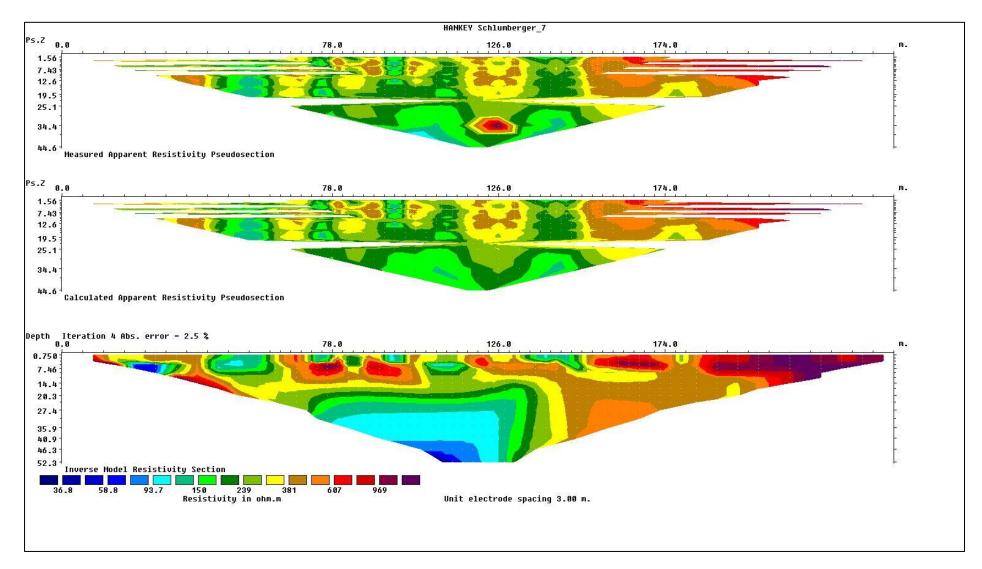


Figure 9 shows the resistivity survey line 2 conducted along a southeast-northwest direction over a 240 m traverse line

5. BOREHOLE DRILLING AND LOGGING RESULTS

5.1. GENERAL

A total of 19 boreholes were drilled at the site (Figure 10). The boreholes were drilled with a rotary percussion drilling rig (Zeus 5000 rig) at 21 bar air pressure with a capacity of 0.519 m³ per second using the Reverse Circulation Method. A 208 mm clay cutter was utilized instead of a drilling bit on portions of the area underlain by clay material and a 145 mm reverse circulation drill bit was used on rock material.

The layout of the drilled boreholes is shown in Figure 10. It should be noted that all boreholes were backfilled with the extracted access sample and sealed with concrete for 1 m.

Chip logging of the boreholes was carried out to identify and to characterise the soil and rock horizons of the site as required by SANS 1936. Appendix III presents the details of the chip logs for the boreholes drilled together with the water levels recorded at least 24 hours after drilling was completed.

5.2. DRILLING SPECIFICATIONS

Drilling was conducted to various depths considering aspects of 60 m maximum depth or until at least 6 m of competent dolomite/solid bedrock was penetrated, whichever comes first and the drilling conditions in instances such as where sample was no longer recoverable or deep cavities. Information such as penetration rate per meter, depth of groundwater strikes, hammer rate, sample loss per meter and air loss per meter was recorded during the drilling of the boreholes. For a detailed view of the information captured during drilling please see drillers logs attached under Appendix IV.

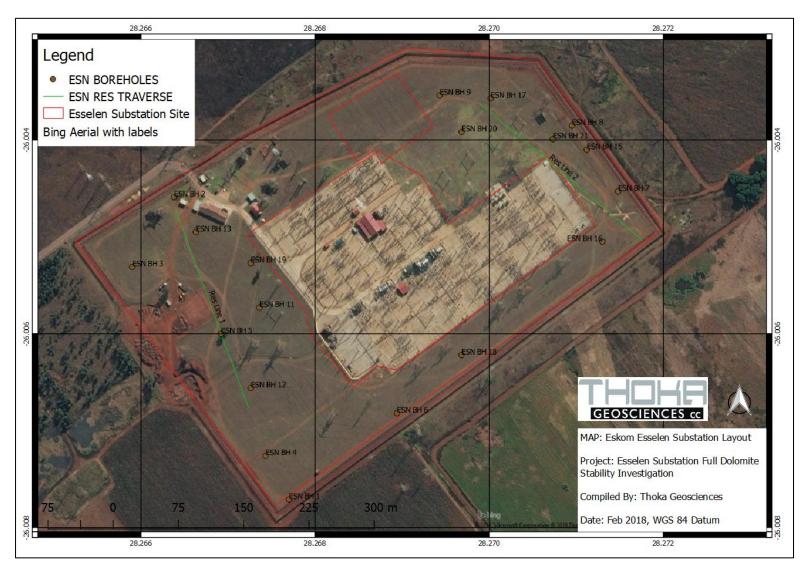


Figure 10 shows the locality of the site, resistivity traverse lines and the drilled boreholes

5.3. SAMPLING AND CHIP LOGGING

Chip samples were collected per meter drilled on site and described according to the current industry standards described in SANS 633. All the samples were put sequentially in plastic sleeves with tags describing the borehole number, penetration rate, and sample depth.

Soil and rock horizons in each of the boreholes were identified and described comprehensively applying the percussion chip logging techniques as required by SANS 1936 and 633. The key sample information recorded for each profile included: Moisture condition, colour, soil texture, origin of soils, rock types, weathering of the rocks and their origin. Appendix III illustrates the results and interpretation of the chip logging conducted and Appendix II shows the Percussion borehole drilling results summary of the boreholes drilled on the site.

A typical logging process would begin with the description of the bulk sample which comments on the bulk moisture, colour and texture of the sample. This is then followed by sieve washed description which includes texture description, colour, rock type and the degree of weathering. The last part of the logging process is the interpretation of the bulk sample description and sieve washed sample description (Appendix III).

5.4. SUMMARY OF PERCUSSION BOREHOLE LOGGING

Borehole logging results indicated occurrence of two groups of geological profiles, one group which is defined by the gravity high zones and higher apparent resistivity signature and the other group defined by gravity low zones and low resistivity signature. The gravity high zones are characterised by shallow dolomite bedrock conditions which vary based on weathered zones of chert and iron oxides/ferruginous soils and cavities. The gravity low zones are characterised by deeper dolomite bedrock overlain by small persistent cavities and soft clay, clayey silt overburden in upwards succession.

Ferruginous Soils

Occurrences of ferruginous soils in the form of SILTY SAND, CLAYEY SILT and ferricrete nodules are encountered at the site to maximum depths of about 7 m from surface. The ferruginous soils are commonly associated with highly weathered CHERT fragments e.g. ESN BH 1, 3, 4, 5, 12, 13, 16, 18 and 21. The occurrence of ferruginous soil is encountered within the gravity high zones of the site associated with shallow bedrock geology.

Transported Soft Clay, Silty Sand,

Soft clay, silty clay material is encountered within the gravity low zones which are associated with deeper dolomite bedrock. The soft clay, silty clay material is typically reddish brown/dark maroon

in colour commonly characterised by rapid penetration rates. The silty clay material is in many cases underlain by persistent cavities until solid dolomite bedrock at depth e.g. ESN BH 15 and ESN BH 8.

Weathered Chert of the Malmani Subgroup

Profiles of moderately to highly weathered CHERT occur commonly within the gravity high zones at the site. The highly weathered CHERT is characterised by rapid penetration rates, and subsequent cavities sometimes in excess of 10 meters e.g. ESN BH 2 and ESN BH 4.

Malmani Subgroup Dolomite

Solid dolomite bedrock is encountered as bluish grey sub-angular chip samples. The solid dolomite bedrock occurs either as shallow bedrock associated with the gravity high zones whereby solid dolomite sometimes rests at 4 meters from the surface. The deep dolomite bedrock is encountered on gravity low zones whereby solid dolomite bedrock can be encountered deeper than 18 m from surface e.g. ESN BH 2 & 3.

5.5. HYDROGEOLOGY OF THE SITE

The assessment of regional groundwater indicates groundwater levels to be at maximum depth of about 30 m from the surface for monitoring station A2N0627.

None of the 19 investigation boreholes at site encountered groundwater. The deepest borehole drilled at site reached depth of 24 m from the surface. This infers a deeper groundwater table at the site and that sinkhole or subsidence associated with groundwater drawdown is unlikely but rather ingress from ponding / percolating surface water is the main trigger for sinkhole formation at the site.

Two additional deeper boreholes were drilled on the northeast portion of the site represented by gravity low conditions in preparation for the dolomite risk management strategy requirements. The boreholes were drilled in access of 30 m from the surface and groundwater was intercepted. But, due to instantaneous collapsing during the drilling process, the boreholes had to be prematurely abandoned. The boreholes proved to be too risky to drill with conventional percussion methods, Thoka Geosciences recommends ODEX drilling systems to be implemented specifically for securing groundwater monitoring points at the site.

6. GEOTECHNICAL SITE APPRAISAL

6.1. GEOTECHNICAL PROVISIONS & LIMITATIONS

A 15 m grid gravity survey was conducted for Esselen Substation prior to commencement of drilling of boreholes.

A total of 21 boreholes were planned as part of Dolomite Stability Investigation with at least 1 or two borehole used for groundwater monitoring purposes. A total of 19 boreholes were drilled at the site for this investigation and two reserved for deeper drilling into identified aquifer zones within the site for monitoring purposes as part of the risk management strategy for the site. The boreholes drilled were to satisfy the requirements for a dolomite stability investigation at Eskom Esselen Substation.

Post drilling of boreholes, a resistivity investigation survey was conducted across parts of the site specifically targeting gravity low and gravity high zones with the aim of mapping the subsurface differences between gravity low and gravity high zones.

6.2. SITE CLASSIFICATION AND DEVELOPMENT POTENTIAL

6.2.1. General IHC Considerations

The purpose of this dolomite stability investigation report is to determine the different engineering geological properties of the surface, subsurface soils and dolomite rocks in accordance with the SANS 1936 guidelines and assign site classification for the dolomite underlain area.

As required by SANS 1936, stability evaluation of the site included the examination of receptacle development potential, mobilising agent, mobilization potential of blanket layer, bedrock morphology and potential sinkhole development space.

Mobilization is defined as the movement of dolomitic overburden by subsurface erosion. Mobilising agents include ingress water, ground vibrations, water level drawdown or any process that can induce mobilisation of material within the blanketing layer under the force of gravity including periodic rise and fall of groundwater level (Buttrick *et al*, 1995).

6.2.2. Groundwater Impact analysis

The site will be susceptible to water ingress from current and future installation of wet services and water ingress from water ponding or percolation from surface runoff.

As discussed above, only two boreholes intercepted groundwater at depths greater than 24 m from surface, the remainder 19 of the boreholes drilled did not encounter groundwater, thus groundwater is believed to be below the cavity prone dolomite horizons at the site (i.e. >24 m

from surface). Thus drawdown of existing groundwater level would have non to little impact on the stability of the dolomitic land as it is not within highly leached / weathered CHERT and DOLOMITE horizons characterised by cavities. This assertion is also reinforced by the fact that groundwater monitoring was conducted within the January – February 2018 period which is expected to be the high rainfall season for the geographical area. Although this is the case, there should be a groundwater management system in place as a precautionary measure to manage the usage/fluctuation of the groundwater particularly focusing on possible rise and fall of the groundwater table that could increase the inherent hazard for ground movement.

According to SANS 1936 - 2, the positive contribution towards dolomite stability of shallow groundwater levels should only be taken into account where:

- The compartmentalization of dolomite groundwater and aerial extent of the compartments have been determined.
- All groundwater users are identified and the maximum quantities abstracted are known.
- There is evidence to show licensed pumping does not exceed normal natural replenishment.
- There are data showing the groundwater level behaviour patterns during periods of drought and recovery.
- The groundwater level is not located within the dolomite residuum.
- The local authority or relevant national department has regularly monitored the groundwater level in the past and continues to do so.

More groundwater monitoring stations must be developed to ascertain the above requirements for groundwater. Thus a groundwater monitoring station at the site is recommended as part of a risk management strategy.

6.2.3. Sinkhole Size Analysis

The method adopted in sinkhole size determination followed the criteria by Buttrick *et al* 2001, whereby bedrock depth shallower than 3 m is expected to be more prone to develop a small (<2 m) sinkhole and bedrock depth >3 m <15 m is more prone to develop a medium sinkhole and bedrock depth deeper 15 m more prone to develop a large sinkhole (5-15 m Wide Sinkholes).

Due to the variable depth to dolomite bedrock, the site is inferred to be more prone to develop medium to large sinkholes respective to the borehole results. Medium sinkholes are sinkholes expected to have a nominal diameter of 2-5 m and large sinkhole expected to have 5-15 m diameter. Based on the assessment of the geophysics, gravity low zones have deeper bedrock

and more prone to develop large sinkholes and gravity high zones are more prone to develop medium to large sinkholes.

6.2.4. Blanketing Material/layer Analysis

The blanketing material at the site may be summed up as ferrugenised SILTY SAND, SILT and SOFT CLAY with fragments of highly weathered CHERT. The geological profiles of the blanketing material at the site although sometimes as thick as 8 m from surface is easily permeable due to the nature and characteristics of the material. Drilling penetration rates recorded to be less 1 minute per meter drilled over the blanketing material suggest moderate to highly permeable material susceptible to collapse/movements. Thus instability triggered by water ingress from the surface is highly likely in situations whereby surface water is not drained at sufficient rate.

6.2.5. Cavities, Voids and Historic Sinkholes/subsidence's

The occurrence of highly weathered chert/dolomite horizons and cavity at shallow and deeper depths as per gravity high and gravity low zones throughout the site suggests typical high inherent hazard for karst formation as voids and cavities form part of receptacles for material of the overlaying strata. Recorded historic sinkhole and subsidence on the western flank of the site further imply high risk zones and caution. The vast area on the western flank of the Eskom Esselen Substation has been zoned of as a high risk area as a whole due to evident sinkhole formation within that area. The triggering mechanism for sinkhole formation is by assessment due to sudden loss of support accelerated by surface water ingress.

6.2.6. Site Classification

Considering all the discussed attributes of mobilization /blanketing material agency, cavities, sinkhole size and nature of blanketing material, dolomite bedrock topography and geohydrology, the site is regarded to have variable hazard classes. Boreholes representing high inherent hazard (IHC 6 – 8) dominate by about 70 % of the drilled borehole classifying either as IHC 6/7/8. Most of the high inherent hazard areas are characterised by either gravity low zones/deep bedrock or contact zones between gravity high and gravity low zones e.g. ESN BH 13. This assessment suggests platforms of shallow dolomite adjacent to troughs of weathered zones covered by soil sediment deposits (FERRUGENISED CLAYEY SILT). The platforms of shallow dolomite in most instances represent intermediate inherent hazard classified as IHC 2/3/4.

Table 1 shows the summary of all the boreholes drilled on site and their inherent hazard classifications.

Chart 3 shows the location of all the boreholes drilled at the site and Chart 3 also shows the inherent hazard classification(s) of the site.

The Dolomite Land Classification for the site is pre-determined by the type of development to be pursued at the site. SANS 1936-1, Table 2 shows the permissible land usage per inherent hazard class for dolomite land. Any other proposed development will thus be guided by the permissible land usage guidelines. The site falls under C3 type development (Electrical Substations, please see

Figure 11) thus developable with footprint requirements up until IHC 6. Therefore any future development at the site should be prioritised on the gravity high zones of the site but with care to avoid the contact zones of gravity high platforms and gravity low troughs. No future development at the site should be conducted or permitted without a detailed footprint investigation.

Portions of the site are thus regarded to be conditionally developable under D3 + Footprint Investigation conditions as a C3 Development Type. Should any future footprint investigations prove worse conditions, the respective area must be regarded as a D4 area with high inherent hazard.

1	2	3	4	5	6	7	8	9	10	
	Land usage	Inherent	Inherent hazard class determined in accordance with the requirements of SANS 1936							
Designation	Description	1	2	3	4	5	6	7	8	
Designation	Description		Dolomite	area desig	nation and	footprint in	vestigation	requiremen	t	
Commercial a	and miscellaneous non-residential usage	•								
C1	Places of detention, police stations, and institutional homes for the handicapped or aged			D3 + FPI				D4		
C2	Hospitals (including clinics), hostels, hotels			D3	+ FPI				D4	
C3	Commercial developments ≤ 3 storeys, including railway stations, shops, wholesale stores, offices, places of worship, theatrical, indoor sports or public assembly venues, other institutional land uses such as universities, schools, colleges, libraries, exhibition halls and museums, light (dry) industrial developments, dry manufacturing, commercial uses such as warehousing, packaging, and electrical substations, filling stations	D2 + FPI	FPI D3 + FPI						D4	
C4	Commercial developments > 3 storeys, including railway stations, shops, wholesale stores, offices, places of worship, theatrical, indoor sports or public assembly venues, other institutional land uses such as universities, schools, colleges, libraries, exhibition halls and museums, light (dry) industrial developments, dry manufacturing, commercial uses such as warehousing, packaging, and electrical substations	D2 + FPI	D3 +FPI		D4					
C5	Fuel depots, processing plants or any other areas for the storage of liquids, waste sites.	D2 + DLI	LI D3+DLI D4							
C6	Outdoor storage facilities, stock yards, container depots	D2 + DLI	2 + DLI D3 + DLI D4				D4			
C7	Parking garages	D2	D2 D3 + FPI D4				D4			
C8	Parking areas	D2				D3			D4	
FPI = Design I NOTE 1 D1,	evel investigation in accordance with the requirements of S. evel investigation specifically below the footprint of the struct D2, D3 and D4 have the meanings assigned in table 1. idential coverage ratio = footprint area/site area.		2, as deeme	ed appropria	ite by the co	mpetent pers	son.			

Figure 11 Extracted from Table 2 of SANS 1936-1; page 11 shows the permissible infrastructure and social facilities per inherent hazard class.

NOTE: Development of any future C3 type development will require a footprint investigation of the proposed development.

Table 1: Summary of Inherent Hazard Class Classifications

Borehole Number	Wad	Voids	Air Loss	Sample Loss	Sinkhole Size Class	Average Mobilization Potential	Sinkhole Hazard	Subsidence Depression. Hazard	Inherent Hazard Class (IHC) for Water ingress Scenario/Non e Dewatering	Inherent Hazard Class (IHC) for Dewatering Scenario
ESN BH 1	None	-	4 - 8	4 - 8	Medium	High	High	High	6	6
ESN BH 2	None	8-19	6 – 19	6 – 19	Large	High	High	High	8	8
ESN BH 3	None	7 – 10 13 - 16	6 – 19	6 - 19	Large	High	High	High	8	8
ESN BH 4	None	6 – 13	3 – 13	6 – 13	Large	High	High	High	8	8
ESN BH 5	None	-	-	-	Small – Medium	Low - Medium	Low - Medium	Medium	2	2
ESN BH 6	None	-	-	-	Small – Medium	Low - Medium	Medium	Medium	3	3
ESN BH 7	None	7 – 10	7 – 11	7 – 12	Medium	High	High	High	6	6
ESN BH 8	None	8-14	8 – 19	8 – 14	Medium	High	High	High	6	6
ESN BH 9	None	6-8	6 – 14	6 – 8	Medium	High	High	High	6	6
ESN BH 10	None									

Borehole Number	Wad	Voids	Air Loss	Sample Loss	Sinkhole Size Class	Average Mobilization Potential	Sinkhole Hazard	Subsidence Depression. Hazard	Inherent Hazard Class (IHC) for Water ingress Scenario/Non e Dewatering	Inherent Hazard Class (IHC) for Dewatering Scenario
ESN BH 11	None	3-5	3 -7	3 - 5	Medium	Medium	High	High	6	6
ESN BH 12	None	-	-	-	Medium	Medium	Small to Medium	Medium	3	3
ESN BH 13	None	-	2 – 10	-	Medium	High	High	High	6	6
ESN BH 14	None									
ESN BH 15	None	8-18	8 – 24	8 – 18	Large	High	High	High	7	7
ESN BH 16	None	9-19	7 – 19	9 – 19	Large	High	High	High	8	8
ESN BH 17	None	6-14	6 – 14	6 – 14	Large	High	High	High	8	8
ESN BH 18	None	-	5 – 6	-	Medium	Medium	Medium	Medium	3/4	3/4
ESN BH 19	None	-	-	-	Medium	Medium	Medium	Medium	3/4	3/4
ESN BH 20	None	-	-	-	Medium	Medium	Medium	Medium	3 / 4	3/4
ESN BH 21	None	9-11	9 - 13	9 – 11 12.5 – 21	Large	High	High	High	7	7

6.3. LAND USE RESTRICTIONS

Table 2 below shows the general requirements for the development of D1, D2, D3 and D4 dolomite area designation. For a detailed view of the precautionary measures required for the D2, D3 and D4 dolomite area designation, please view the extracts from SANS 1936-3. As these are very comprehensive and detailed, it will suffice to say that these foundation design and water precautionary related measures cover the proposed new development type, as well as their future upgrading and maintenance.

Table 2: Shows a general overview of the requirements for D2, D3 and D4 dolomite land.

Dolomite area designation	Description
D1	No precautionary measures are required.
D2	General precautionary measures, in accordance with the requirements of SANS 936-3, that are intended to prevent the concentrated ingress of water into the ground, are required.
D3	Precautionary measures in addition to those pertaining to the prevention of concentrated ingress of water into the round, in accordance with the relevant requirements of SANS 1936-3 are required.
D4	The precautionary measures required in terms of SANS 1936-3 are unlikely to result in a tolerable hazard. Sitespecific precautionary measures are required.

6.4. Precautionary Measures For Eskom Esselen Substation

The development on the site needs to adhere to several precautionary measures as stipulated in SANS 1936-3:2012 and these include:

 No ponding of surface water should be able to occur adjacent to foundations both during and after construction

- Storm water should be effectively captured and led well away from all structures.
- All wet services should be flexible in design and should specifically be designed to accommodate movement where entering or leaving structures.
- All foundations should be inspected by a competent person to ensure that the desired founding medium has been attained and that recommendations made in this report have been adhered to.
- Brick force should be incorporated in all layers of the plinth wall for load bearing brickwork and in every fourth layer. At least three layers of brick force should be placed above all openings such as doors.
- All building on site shall be constructed in accordance with relevant codes of practice.
- Sinkhole manifestation is expected to be in the form of medium to large sinkholes. The
 notional sinkhole diameter at the site is thus deduced to be equals to or greater than 5 m in
 diameter.
- Development on D3 + FPI sites should be constructed in such a manner that a sinkhole or subsidence that has a likelihood of occurring and that has the notional diameter of surface manifestation indicated in the geotechnical report within or adjacent to the footprint of such building, shall not result in the toppling or sliding failure of the building or a portion thereof into such a sinkhole or subsidence (SANS 1936-3:2012).
- Thus all new structures developed should be designed to span at least 5 15 m loss of support
- An engineered soil raft / raft should be used in areas of shallow dolomite bedrock. This mattress has the dual purpose of improving founding conditions. The construction of such a mattress involves the removal and replacement of unsuitable soil beneath and for 3 m beyond the periphery of buildings. The precise design of the mattress will be dependent on bedrock depth and the nature of the local soil materials.

- Storm water upstream of building and structures shall be diverted away from the building or structure in such a manner that concentration of storm water is minimized or led to a storm water drainage system.
- According to SANS 1936, in addition to the above mentioned precautionary measures, sites
 which fall under D4 dolomite land need to adhere to site-specific precautionary measures.
- According to the guidelines for consultants for development of infrastructure on dolomite (Department of Public Works, 2004). High risk areas should adhere to precautionary measures as shown below:
- Use only HDPE piping for water (Class PN 12, 5 and higher), sewer (Class PN 10 minimum) and storm water (8 kN/m² ring stiffness) in 21 high-risk areas or where services traverse high-risk areas. All material for HDPE pipes, structures and fittings must be in accordance with SABS ISO 4427/SANS 4427 for type PE 100 and all welding and manufacturing to be in accordance with SABS 0268/SANS 10268, SABS 0269/SANS 10269, SABS 0270/SANS 10270, SABS 1655/SANS 1655 and SABS 1671/SANS 1671 codes.
- In extremely problematic areas water reticulation may be placed above ground or all services may be placed in ducts or sleeves where within fifteen metres of a building. Sleeves to be provided with inspection chambers at both ends and must comply with the requirements of a sewer system for high risk areas. All sleeve systems must be constructed to designed slopes that permit drainage to predetermined inspection manholes.
- Ablution facilities should not be included in the principal buildings or infrastructure. These
 facilities should be isolated in such a manner as to avoid damage to other parts of the
 development in the event of service failure and sinkhole/doline formation.
- Use aprons of large (5.0 m width) impervious paved areas around structures to enhance drainage. If rapid drainage (slopes 1:15 and steeper) away from structures is possible apron slabs may be reduced to 3.0 m width.

- Blanketing of geotechnical problematic areas with impervious material (clayey soil and/or HDPE sheeting), concrete or paving bricks need to be introduced if such areas could influence the structural integrity of buildings.
- Contouring of site to achieve a fall of least 1:40 in general and to 1:15 away from structures within a distance of 8 m of a structure is required.

6.5. DOLOMITE RISK MANAGEMENT PLAN & RECOMMENDATIONS

A crucial requirement of development on dolomitic lands, as stipulated in SANS 1936-1, is the establishment and implementation of a dolomite risk management strategy (DRMS) by the developer, which must tie in with that in place and controlled by the relevant local authority which in this regard the relevant authority would be the Ekurhuleni Municipality. SANS 1936-4 describes this in detail.

It is therefore highly recommended that Thoka Geosciences be appointed to develop a dolomite risk management strategy so as to minimize further development of sinkholes within the station. The dolomite risk management strategy will entail amongst others, monitoring regional groundwater, monitoring local groundwater fluctuations, monitoring of surface flow on site and emergency preparedness plan in cases where sinkholes develop.

The risk management strategy will be tied with local municipalities risk management strategy in place also providing reporting strategies of sinkhole and subsidence events. The training of local personnel occupying the Esselen Substation will be part and parcel of the risk management strategy. Training will include amongst other aspects, training of personnel on identification of sinkholes, subsidence, tension cracks leading to sinkholes, and management of surface water ponding and reporting strategies.

Urgency to rehabilitate existing sinkholes must be placed and further assessment of the dolomite stability beyond the outer parameter fence onto the boundary of Eskom Esselen Substation servitude needs to be prioritised as a matter of urgency.

Areas deduced to be IHC 8 from this investigation are recommended for proactive rehabilitation in the form of overburden strip, expose cavity throat and rehabilitation to prevent highly likely sinkhole occurrences.

7. CONCLUSIONS

Due to the variable depth to dolomite bedrock, the site is inferred to be more prone to develop medium to large sinkholes respective to the borehole results. Medium sinkholes are sinkholes expected to have a nominal diameter of 2-5 m and large sinkhole expected to have 5-15 m diameter. Based on the assessment of the geophysics, gravity low zones have deeper bedrock and more prone to develop large sinkholes and gravity high zones are more prone to develop medium to large sinkholes.

The blanketing material at the site may be summed up as ferrugenised SILTY SAND, SILT and SOFT CLAY with fragments of highly weathered CHERT. The geological profiles of the blanketing material at the site although sometimes as thick as 8 m from surface is easily permeable due to the nature and characteristics of the material. Drilling penetration rates recorded to be less 1 minute per meter drilled over the blanketing material suggest easily permeable material susceptible to collapse/movements. Thus instability triggered by water ingress from the surface is highly likely in situations whereby surface water is not drained at sufficient rate.

The occurrence of highly weathered chert/dolomite horizons and cavity at shallow and deeper depths as per gravity high and gravity low zones throughout the site suggests typical high inherent hazard for karst formation as voids and cavities form part of receptacles for material of the overlaying strata. Recorded historic sinkhole and subsidence on the western flank of the site further imply high risk zones and caution. The vast area on the western flank of the Eskom Esselen Substation has been zoned of as a high risk area as a whole due to evident sinkhole formation within that area. The triggering mechanism for sinkhole formation is by assessment due to sudden loss of support accelerated by surface water ingress.

Considering all the discussed attributes of mobilization /blanketing material agency, cavities, sinkhole size and nature of blanketing material, dolomite bedrock topography and geohydrology, the site is regarded to have variable hazard classes. Boreholes representing high inherent hazard (IHC 6 – 8) dominate by about 70 % of the drilled borehole classifying either as IHC 6/7/8. Most of the high inherent hazard areas are characterised by either gravity low zones/deep bedrock or contact zones between gravity high and gravity low zones e.g. ESN BH 13. This assessment suggests platforms of shallow dolomite adjacent to troughs of weathered zones covered by soil sediment deposits (FERRUGENISED CLAYEY SILT). The

platforms of shallow dolomite in most instances represent intermediate inherent hazard classified as IHC 2/3/4.

The Dolomite Land Classification for the site will be pre-determined by the type of development to be pursued at the site. SANS 1936-1, Table 2 shows the permissible land usage per inherent hazard class for dolomite land. Any proposed development will thus be guided by the permissible land usage guidelines. The site falls under C3 type development, thus developable with footprint requirements up until IHC 6. Therefore any future development at the site should be prioritised on the gravity high zones of the site but with care to avoid the contact zones of gravity high platforms and gravity low troughs. No future development at the site should be conducted or permitted without a detailed footprint investigation.

Portions of the site are thus regarded to be conditionally developable under D3 + Footprint Investigation conditions as a C3 Development Type. Should any future footprint investigations proof worse conditions, the respective area must be regarded as a D4 area with high inherent hazard.

A crucial requirement of development on dolomitic lands, as stipulated in SANS 1936-1, is the establishment and implementation of a dolomite risk management strategy (DRMS) by the developer, which must tie in with that in place and controlled by the relevant local authority which in this regard the relevant authority would be the Ekurhuleni Municipality. SANS 1936-4 describes this in detail.

It is therefore highly recommended that Thoka Geosciences be appointed to develop a dolomite risk management strategy so as to minimize further development of sinkholes within the station. The dolomite risk management strategy will entail amongst others, monitoring regional groundwater, monitoring local groundwater fluctuations, monitoring of surface flow on site and emergency preparedness plan in cases where sinkholes develop.

The risk management strategy will be tied with local municipalities risk management strategy in place also providing reporting strategies of sinkhole and subsidence events. The training of local personnel occupying the Esselen Substation will be part and parcel of the risk management strategy. Training will include amongst other aspects, training of personnel on identification of sinkholes, subsidence, tension cracks leading to sinkholes, and management of surface water ponding and reporting strategies.

Dolomite Stability Investigation Report: Eskom Esselen Substation. Report by: Thoka Geosciences, PO Box 1910, Silverton, 0127. **Thokageo@gmail.com** Cell: +27 11 412 1449 Fax: 0865168363

Urgency to rehabilitate existing sinkholes must be placed and further assessment of the dolomite stability beyond the outer parameter fence onto the boundary of Eskom Esselen Substation servitude needs to be prioritised as a matter of urgency.

Areas deduced to be IHC 8 from this investigation are recommended for proactive rehabilitation in the form of overburden strip, expose cavity throat and rehabilitation to prevent highly likely sinkhole occurrences.

REPORT SIGN OFF

Tshwaro ME Mogadime Pr.Sci.Nat.

Managing Director

PROJECT TEAM

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Tseliso Lebasa BSc Env & Engineering Geology

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9. APPENDICES

Appendix I

FIGURES

Chart 1: Locality and Layout Map

Chart 2: Geological Map 2628 East Rand

Chart 3: Inherent Hazard Class Zonation

Appendix II

Borehole Summary Table

Appendix III

Percussion Borehole Profile Log Sheets

Appendix IV

Drillers Logs

Appendix V

Gravity Survey Results

Appendix VI

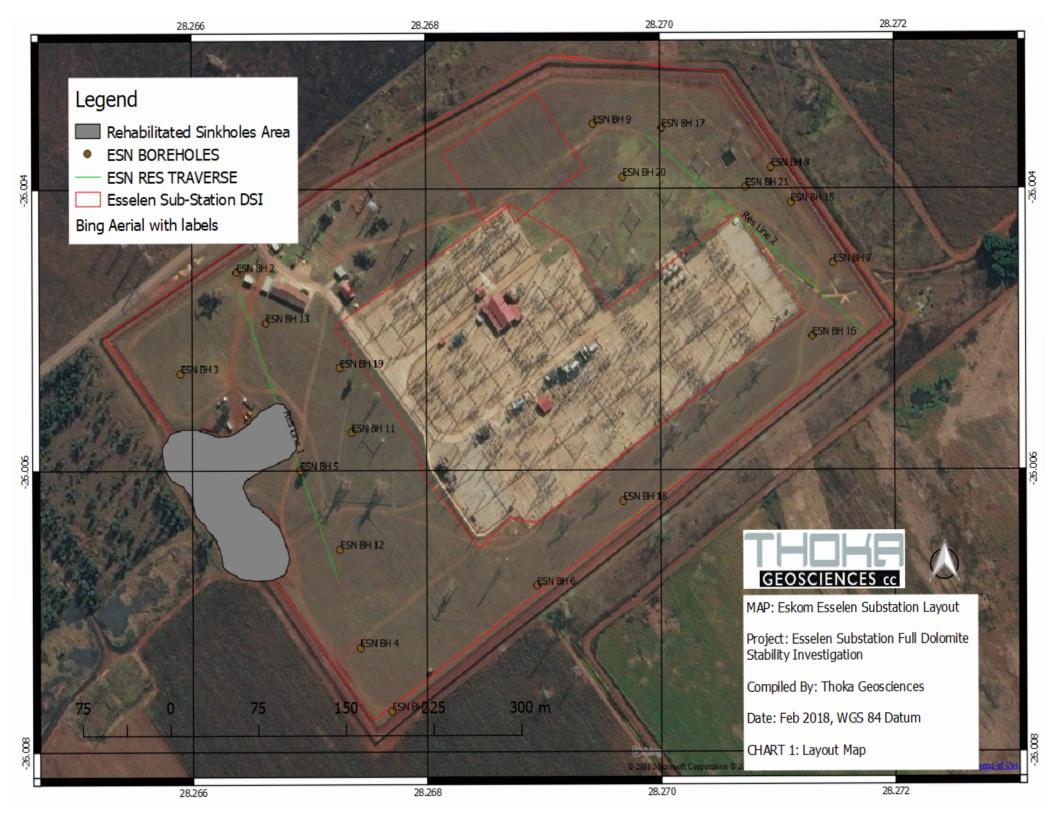
Council for Geosciences Review

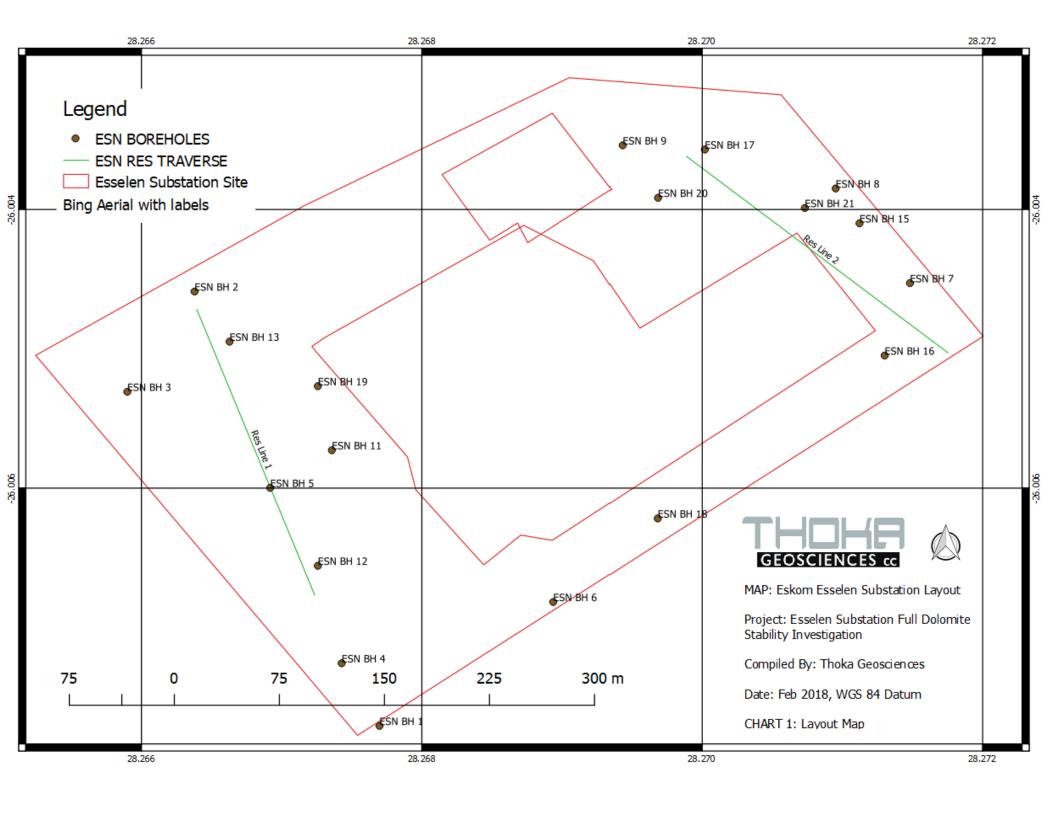
Appendix I

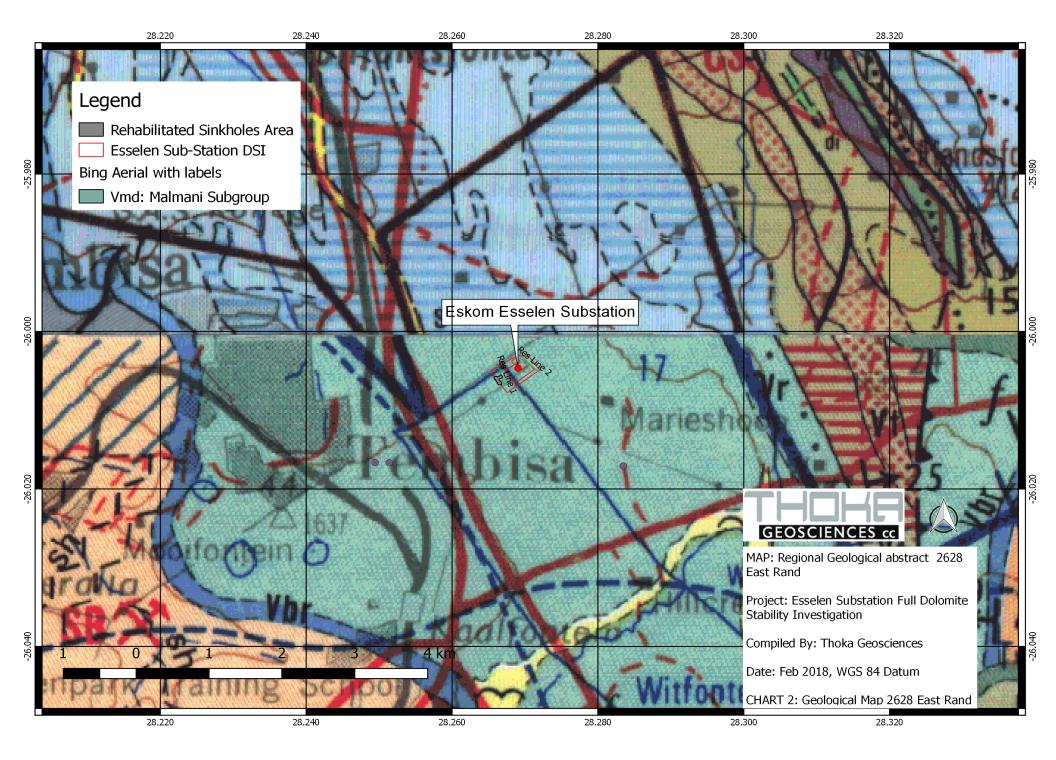
Chart 1: Locality and Layout Map

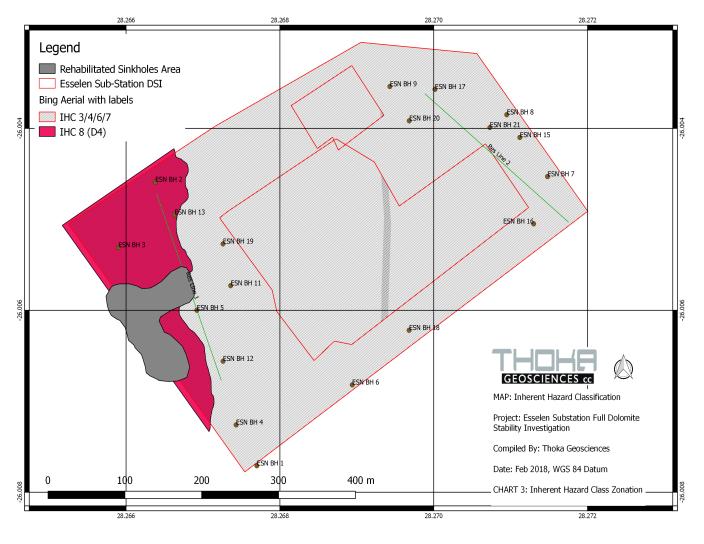
Chart 2: Geological Map 2628 East Rand

Chart 3: Inherent Hazard Class Zonation









Appendix II

Borehole Summary Table

Borehole Number	Borehole Coordinates	Ferricrete Soil with Highly Weathered Chert Fragments / Transported Soils	Highly to Moderately Weathered Chert and Dolomite	Moderately - Slightly Weathered Chert & Dolomite	Cavity / Voids	Solid Unweathered Dolomite Rock/Bedrock	Depth to Water Table Levels after 24 Hours	Inherent Hazard Class - Water Ingress Scenario	Inherent Hazard Class - Dewatering Scenario
ESN BH 1	28.26770 26.00771	0 – 7 m: Ferricrete Soil with Fragments of Highly Weathered CHERT		8 – 12 m: Moderately Weathered CHERT		12 – 18 m: Solid Unweathered Dolomite	Dry	6	6
ESN BH 2	28.26638 26.00459	0 – 4 m: Ferruginous SILTY SAND	4 – 8 m: Moderately weathered CHERT		8 – 19 m: Cavity		Dry	8	8
ESN BH 3	28.26590 26.00531	0 – 3 m: Highly Weathered Chert & Ferricrete soils	3 – 7 m: Highly weathered Chert 10 – 13 m: Highly weathered CHERT	16 – 18 m: Highly to Slightly weathered Dolomite	7 – 10 m: Rapid Penetration Rate (Cavities) 13 – 16 m: Rapid Penetration Rate (Cavities)	18 – 24 m: Solid Dolomite	Dry	8	8
ESN BH 4	28.26743 26.00726	0 – 1 m: Ferricrete soil with fragments of Highly weathered CHERT	1 – 6 m: Highly weathered CHERT		6 – 13 m: Rapid Penetration Rate (Cavities)		Dry	8	8
ESN BH 5	28.26692 26.00600	0 – 2 m: Ferricrete soil with fragments of Highly weathered CHERT		2 – 4 m: Slightly weathered Dolomite		4 – 10 m: Solid Dolomite	Dry	2	2
ESN BH 6	28.26894 26.00682	0 – 3 m: Ferricrete soil		3 – 4 m: Moderately weathered Dolomite		4 – 10 m: Solid Dolomite	Dry	3	3
ESN BH 7	28.27148 26.00453	0 – 7 m: Clayey SILT		10 – 13 m: Moderately weathered DOLOMITE and CHERT	7 – 10 Rapid Penetration Rate (Cavities)	13 – 19 m: Solid DOLOMITE	Dry	6	6
ESN BH 8	28.27095 26.00385	0 – 8 m: Soft CLAY			8 – 14 m: Rapid Penetration Rate (Cavities)	14 – 20 m: Solid DOLOMITE	Dry	6	6
ESN BH 9	28.26943 26.00354	0 1 m: Colluvium Material	1 – 6 m: Moderately weathered CHERT		6 – 8 m: Cavities	8 – 14 m: Solid Dolomite	Dry	6	6
ESN BH 11	28.26736 26.00573	0 – 2 m: Ferruginsed soil with Highly weathered CHERT fragments	2 – 3 m: Highly weathered CHERT 5 – 8 m: Highly to moderately weathered CHERT and DOLOMITE		3 – 5 m: Rapid Penetration Rate (Cavities)	8 – 14 m: Solid Dolomite	Dry	6	6

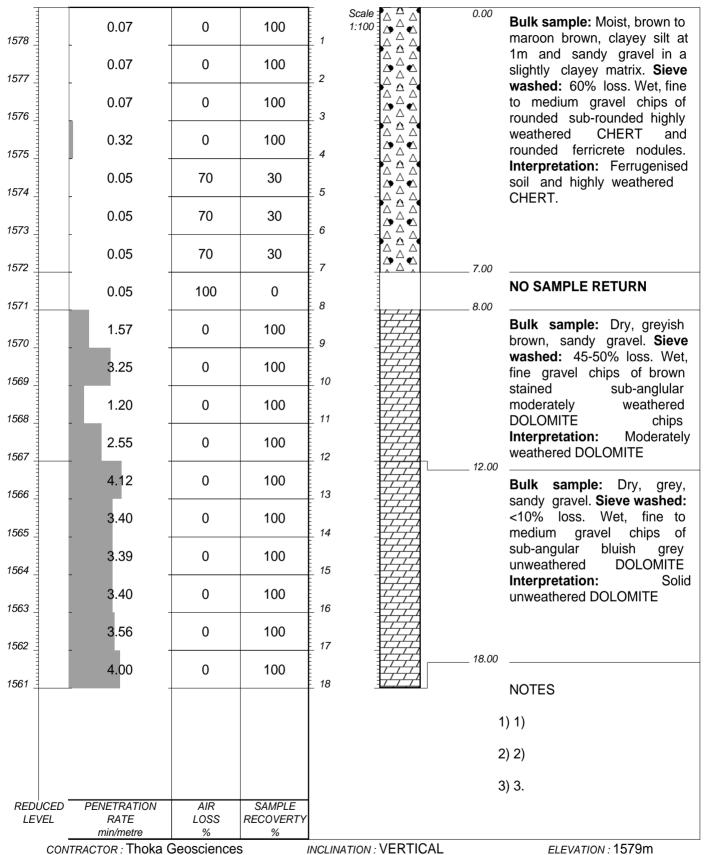
Borehole Number	Borehole Coordinates	Ferricrete Soil with Highly Weathered Chert Fragments / Transported Soils	Highly to Moderately Weathered Chert and Dolomite	Moderately - Slightly Weathered Chert & Dolomite	Cavity / Voids	Solid Unweathered Dolomite Rock/Bedrock	Depth to Water Table Levels after 24 Hours	Inherent Hazard Class - Water Ingress Scenario	Inherent Hazard Class - Dewatering Scenario
ESN BH 12	28.26726 26.00656	0 – 3 m: Ferruginsed soil with Highly weathered CHERT fragments		3 – 6 m: Moderately weathered DOLOMITE		6 – 12 m: Solid DOLOMITE	Dry	3	3
ESN BH 13	28.26663 26.00495	0 – 3 m: Highly Weathered CHERT with Ferricrete nodules	3 – 9 m: Highly Weathered CHERT			9 – 15 m: Solid DOLOMITE	Dry	6	6
ESN BH 15	28.27112 26.00410	0 – 8 m: Silty Clay			8 – 18 m: Cavities with hanging boulders at 9 – 15 m	18 – 24 m: Solid Dolomite	Dry	7	7
ESN BH 16	28.27130 26.00505	0 – 3 m: Highly weathered CHERT in ferruginised SAND matrix	3 – 9 m: Highly weathered CHERT and DOLOMITE		9 – 19 m: Cavities		Dry	8	8
ESN BH 17	28.270016 26.00357	0 – 6 m: CHERT in a SILTY CLAY matrix			6 – 14 m: Cavities		Dry	8	8
ESN BH 18	28.26968 26.00622	0 – 7 m: Highly weathered CHERT		7 – 11 m: Slightly weathered CHERT and DOLOMITE		11 – 17 m: Solid DOLOMITE	Dry	3/4	3/4
ESN BH 19	28.26726 26.00527	0 – 2 m: HILLWASH	2 – 7 m: Moderately weathered CHERT	7 – 11 m: Moderately weathered DOLOMITE		11 – 17 m: Solid DOLOMITE	Dry	3/4	3/4
ESN BH 20	28.269682 -26.003917	0 – 2 m: Highly weathered CHERT	2 – 3 m: Highly weathered CHERT and DOLOMITE			3 – 9 m: Solid DOLOMITE	Dry	2/3	2/3
ESN BH 21	28.27073 26.00399	0 – 5 m: Clayey Ferruginous clayey SILT and ferruginous Gravel	5 - 9 m: Moderately weathered CHERT and DOLOMITE	11 – 12.5 m: Moderately weathered to slightly weathered DOLOMITE	9 – 11 m: Cavity 12.5 – 15 m: Cavity	15 – 21 m: NO SAMPLE return but drilling indicates solid bedrock conditions	Dry	7	7

Appendix III

Percussion Borehole Profile Log Sheets

HOLE No: 1 Sheet 1 of 1

JOB NUMBER: ESN BH 1



MACHINE: THOR 5000

DRILLED BY:

PROFILED BY: Melusi Mashego

TYPE SET BY:

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DIAM: DATE .

DATE: 15 JANUARY 2018

DATE: 05/02/2018 15:23

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ELEVATION: 1579m X-COORD: 28.26770 Y-COORD: 26.00771

> HOLE No: 1 Esselen Substation, Tembisa



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 1

DOLOMITE	{SA13}
CHERT	{SA21}
FERRICRETE NODULES	{SA24}

CONTRACTOR:INCLINATION:ELEVATION:MACHINE:DIAM:X-COORD:DRILLED BY:DATE:Y-COORD:

DATE:

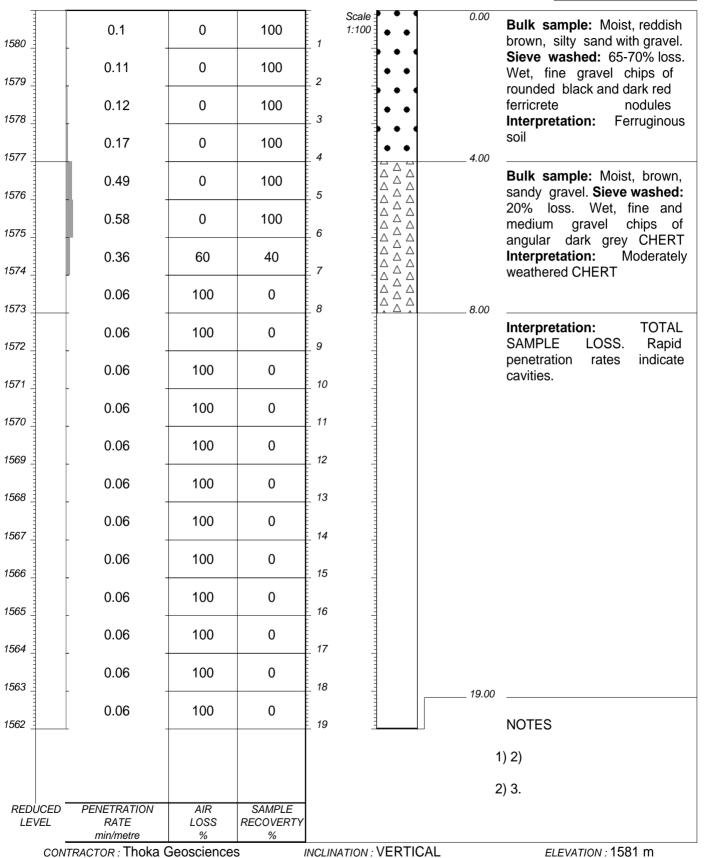
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LEGENDSUMMARY OF SYMBOLS

HOLE No: 02 Sheet 1 of 1

JOB NUMBER: ESN BH 2



MACHINE: THOR 5000

DRILLED BY:

PROFILED BY: Melusi Mashego

TYPE SET BY:

SETUP FILE: STANDARD.SET

DIAM:

DATE: 10 JANUARY 2018

DATE: 05/02/2018 15:26

TEXT: ..I\ProfileLogs\ESNBH2.TXT

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> HOLE No: 02 Esselen Substation, Tembisa



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 2

	CHERT	{SA21}
•	FERRICRETE NODULES	{SA24}

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DATE:

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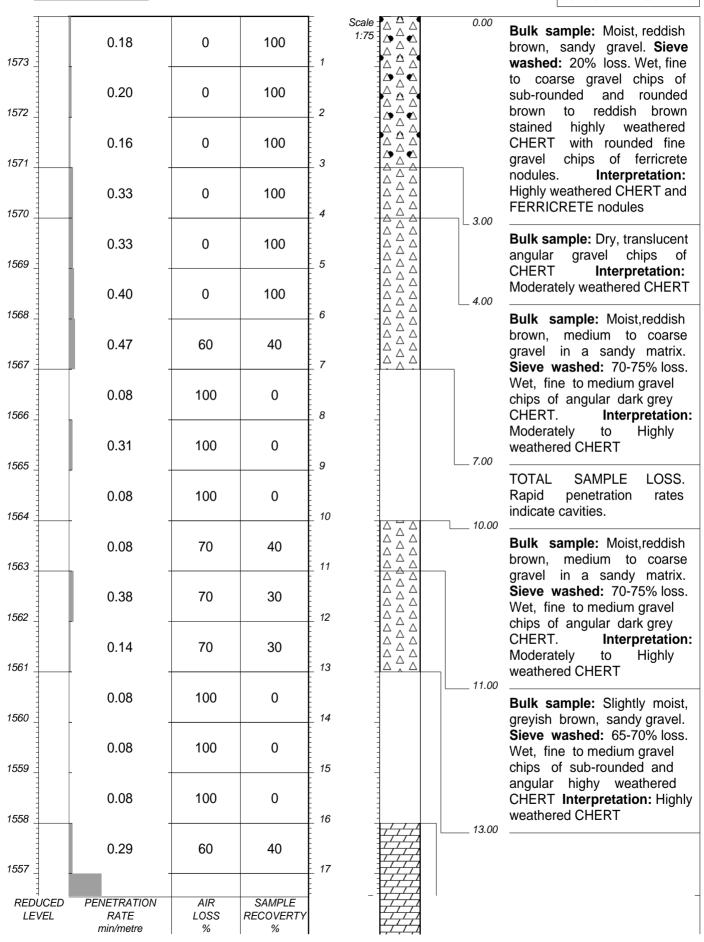
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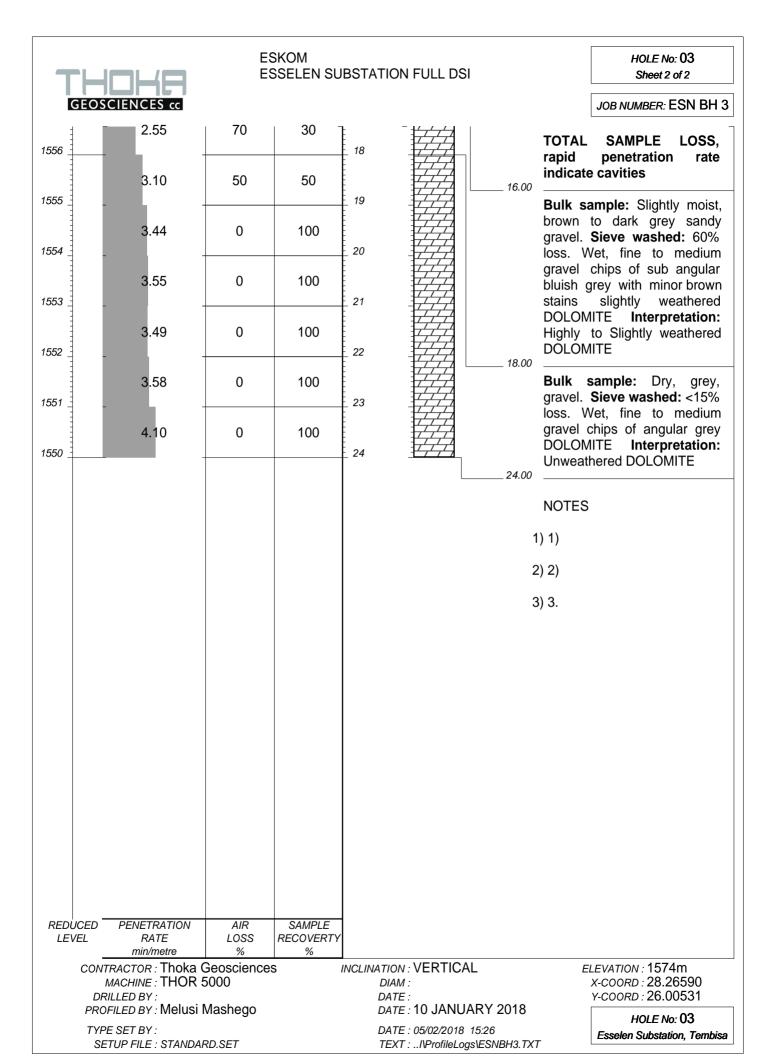
THOHA

ESKOM ESSELEN SUBSTATION FULL DSI

HOLE No: 03 Sheet 1 of 2

JOB NUMBER: ESN BH 3





DOAA Thoka GeoSciences dotPLOT 7016 PBpH67



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 3

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CHERT	{SA21}
FERRICRETE NODULES	{SA24}

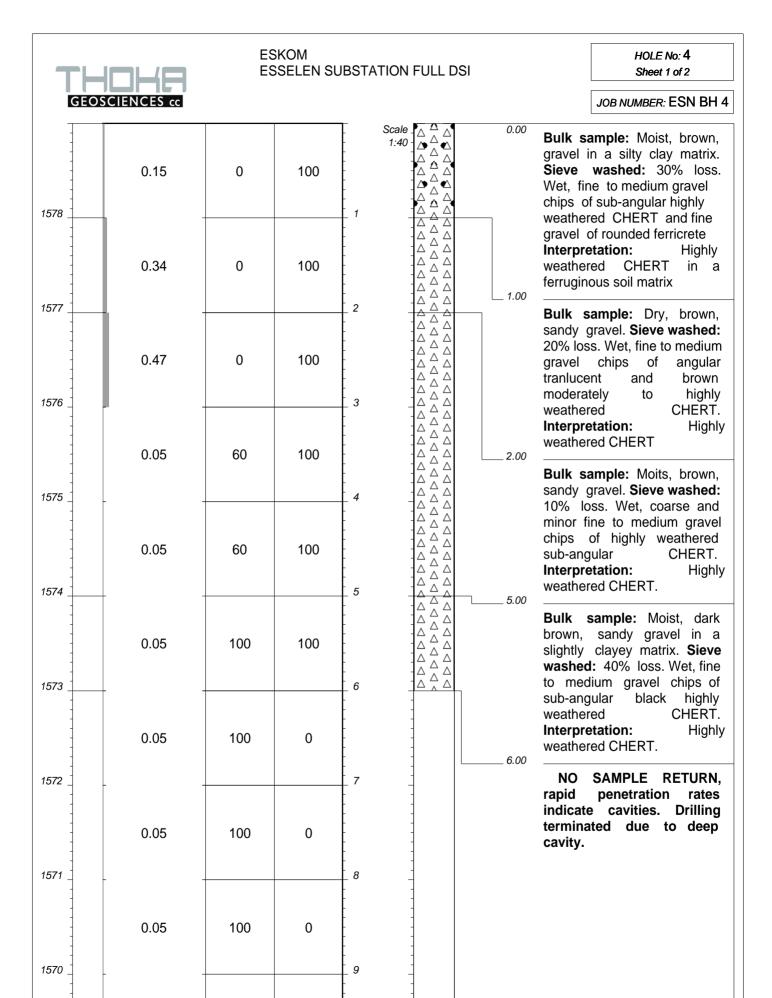
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LEGEND SUMMARY OF SYMBOLS



REDUCED

LEVEL

PENETRATION

RATE

min/metre

SAMPLE

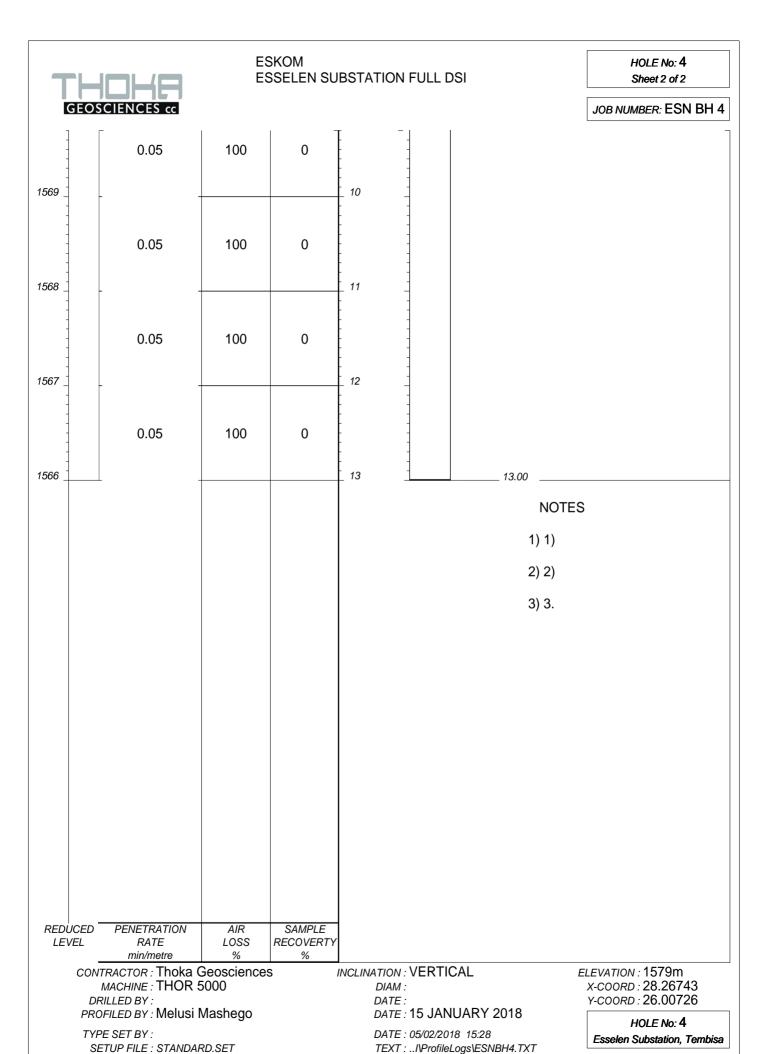
RECOVERTY

%

AIR

LOSS

%





LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 4

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• •	FERRICRETE	{SA24}

CONTRACTOR: INCLINATION: ELEVATION:

MACHINE: DIAM: X-COORD:

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DATE:

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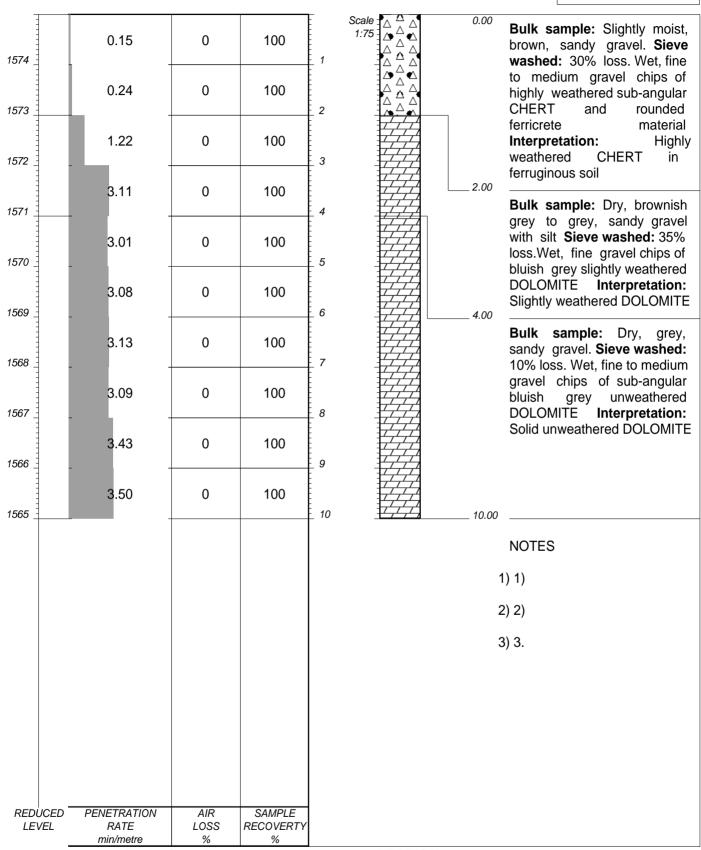
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LEGEND SUMMARY OF SYMBOLS



HOLE No: 5 Sheet 1 of 1

JOB NUMBER: ESN BH 5



CONTRACTOR: Thoka Geosciences

MACHINE: THOR 5000

DRILLED BY :

PROFILED BY: Melusi Mashego

TYPE SET BY :

SETUP FILE: STANDARD.SET

INCLINATION : VERTICAL

DIAM :

DATE: 12 JANUARY 2018

DATE: 05/02/2018 15:29

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ELEVATION: 1575m X-COORD: 28.26692 Y-COORD: 26.00600

HOLE No: 5
Esselen Substation, Tembisa



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 5

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CHERT	{SA21}
FERRICRETE	{SA24}

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DATE:

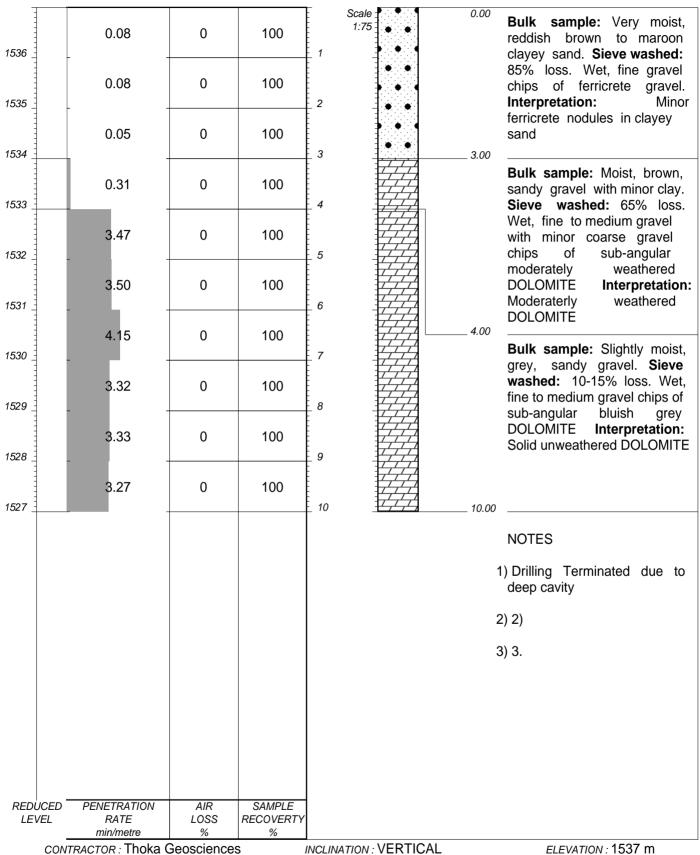
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LEGENDSUMMARY OF SYMBOLS

HOLE No: 6 Sheet 1 of 1

JOB NUMBER: ESN BH 6



MACHINE: THOR 5000

DRILLED BY:

PROFILED BY: Melusi Mashego

TYPE SET BY:

SETUP FILE: STANDARD.SET

DIAM: DATE:

DATE: 15 JANUARY 2018

DATE: 05/02/2018 15:31

TEXT: ..I\ProfileLogs\ESNBH6.TXT

X-COORD: 28.26894 Y-COORD: 26.00682

HOLE No: 6 Esselen Substation, Tembisa



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 6

SAND	{SA04}
DOLOMITE	{SA13}
FERRICRETE	{SA24}

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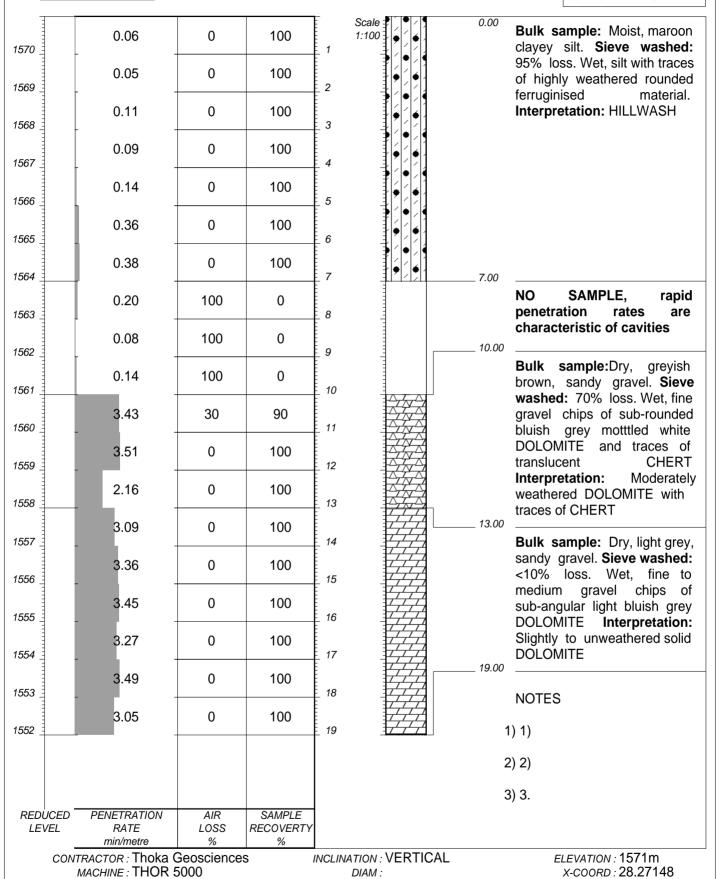
LEGEND SUMMARY OF SYMBOLS

THOMES CC

ESKOM ESSELEN SUBSTATION FULL DSI

HOLE No: 7 Sheet 1 of 1

JOB NUMBER: ESN BH 7



DATE: 16 JANUARY 2018

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DATE: 05/02/2018 15:36

PROFILED BY: Melusi Mashego

SETUP FILE: STANDARD.SET

DRILLED BY:

TYPE SET BY:

Y-COORD: 26.00453

HOLE No: 7

Esselen Substation, Tembisa



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 7

	SILT	{SA06}
	CLAYEY	{SA09}
	DOLOMITE	{SA13}
	CHERT	{SA21}
• •	FERRICRETE	{SA24}

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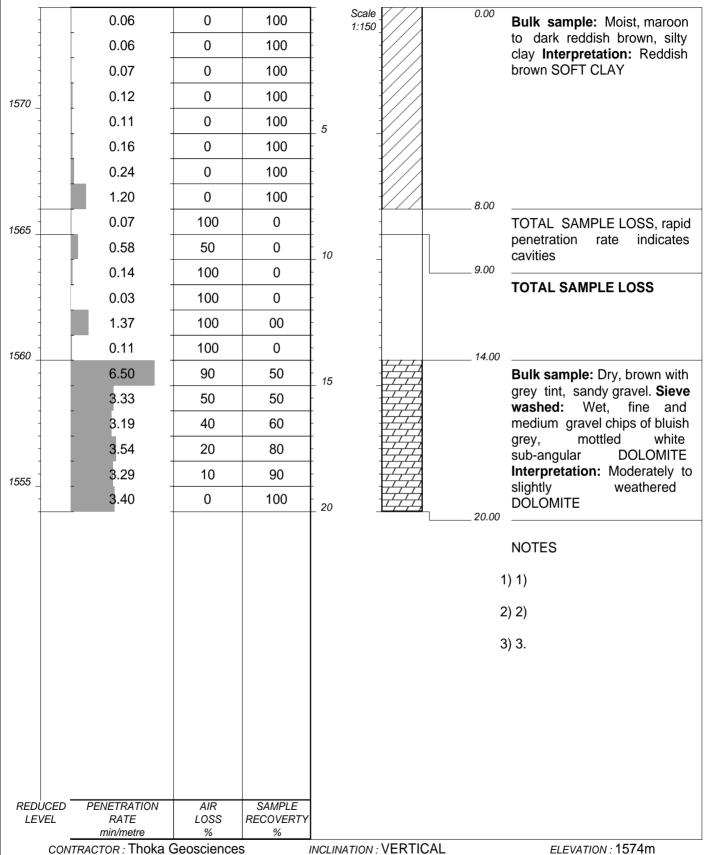
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LEGEND SUMMARY OF SYMBOLS

HOLE No: 08 Sheet 1 of 1

JOB NUMBER: ESN BH 8



MACHINE: THOR 5000

DRILLED BY:

PROFILED BY: Melusi Mashego

TYPE SET BY:

SETUP FILE: STANDARD.SET

DIAM:

DATE: DATE: 17 JANUARY 2018

DATE: 05/02/2018 15:38

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ELEVATION: 1574m X-COORD: 28.27095 Y-COORD: 26.00385

> HOLE No: 08 Esselen Substation, Tembisa



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 8

CLAY	{SA08}
DOLOMITE	{SA13}

CONTRACTOR: INCLINATION: ELEVATION:

MACHINE: DIAM: X-COORD:

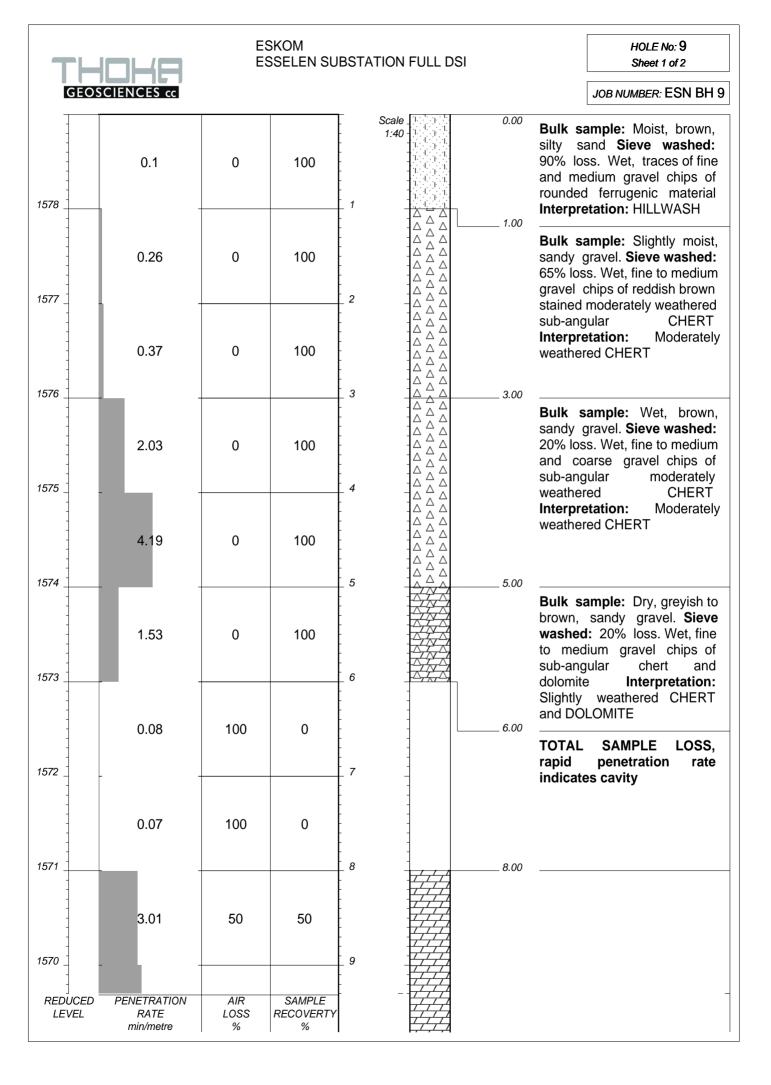
DRILLED BY: DATE: Y-COORD:

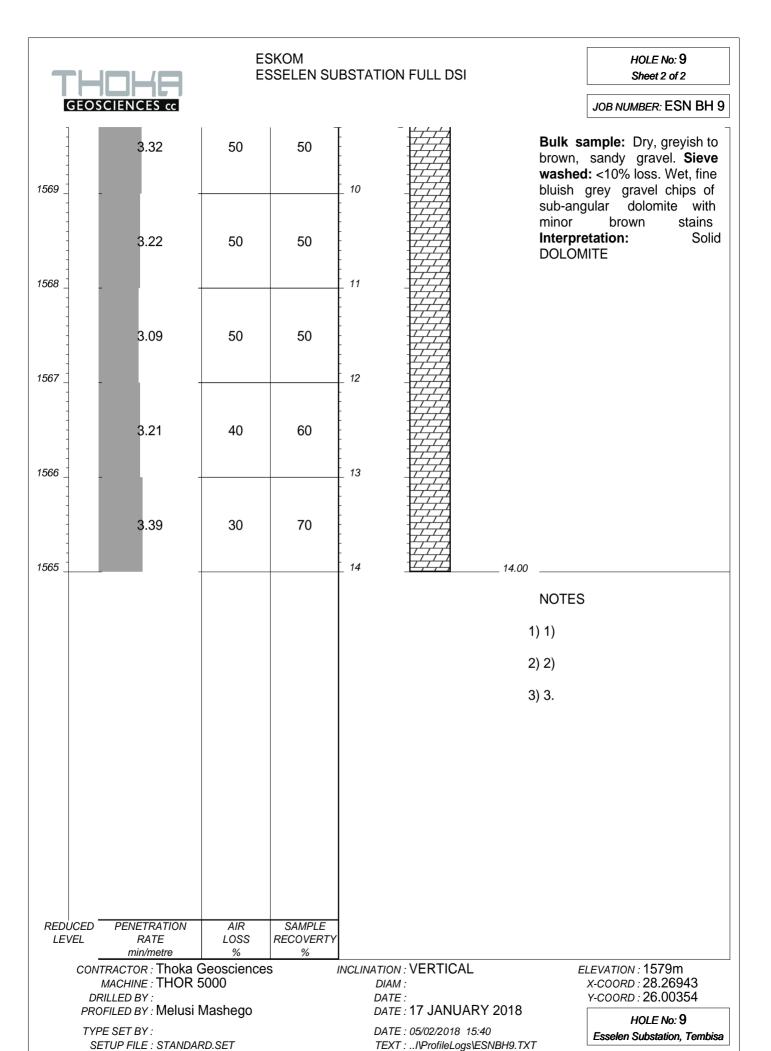
DATE:

TYPE SET BY: DATE: 05/02/2018 15:38

SETUP FILE: STANDARD.SET TEXT: ...I\ProfileLogs\ESNBH8.TXT

LEGEND SUMMARY OF SYMBOLS





DOAA Thoka GeoSciences dotPLOT 7016 PBpH67



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 9

	SAND	{SA04}
	SILTY	{SA07}
7 / / 7 / / 7 / / 7 / /	DOLOMITE	{SA13}
	CHERT	{SA21}

CONTRACTOR:INCLINATION:ELEVATION:MACHINE:DIAM:X-COORD:DRILLED BY:DATE:Y-COORD:

DATE:

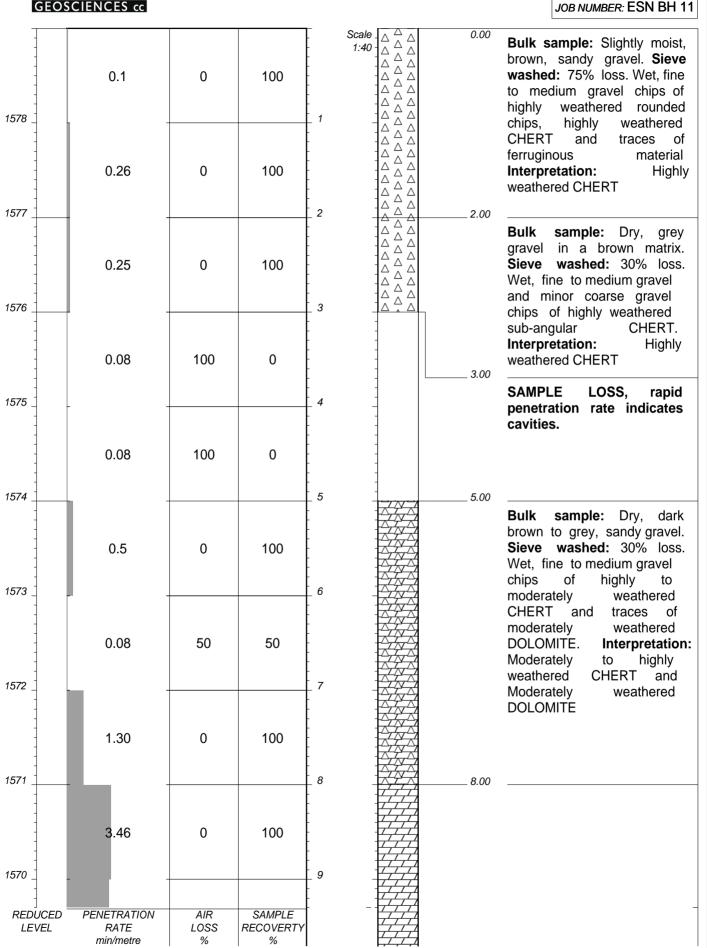
TYPE SET BY: DATE: 05/02/2018 15:40

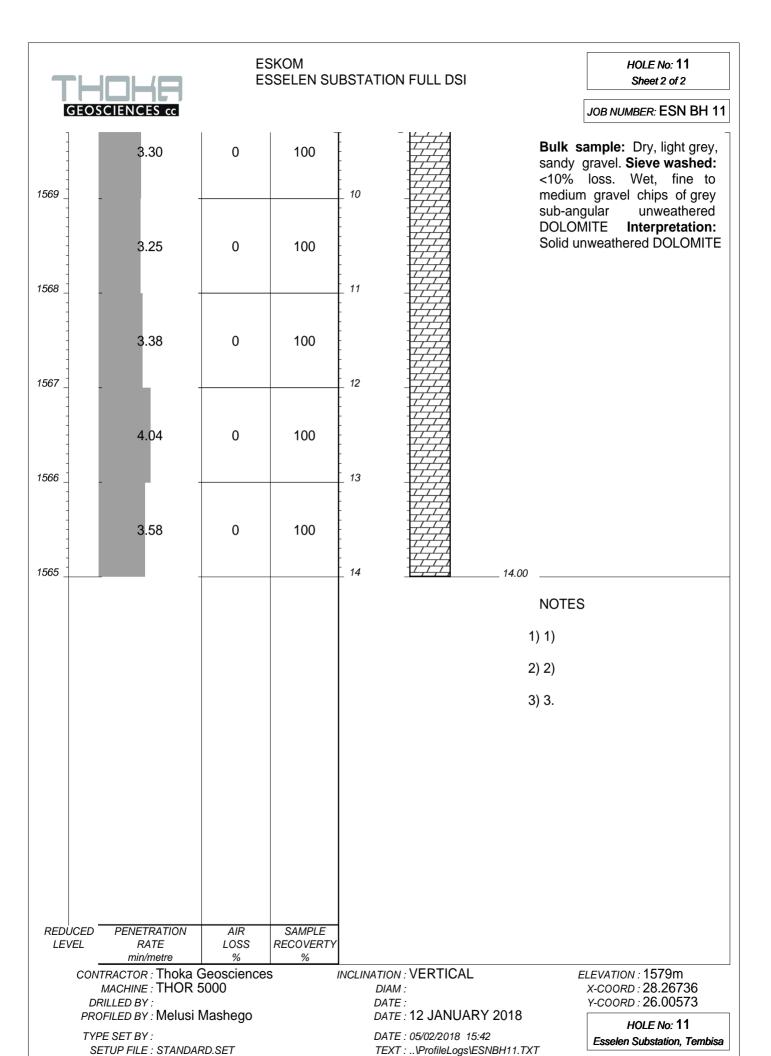
SETUP FILE: STANDARD.SET TEXT: ...I\ProfileLogs\ESNBH9.TXT

LEGEND SUMMARY OF SYMBOLS



HOLE No: 11 Sheet 1 of 2





DOAA Thoka GeoSciences dotPLOT 7016 PBpH67



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 11

/ / / / / / / / / / / /	DOLOMITE	{SA13}
	CHERT	{SA21}

 CONTRACTOR:
 INCLINATION:
 ELEVATION:

 MACHINE:
 DIAM:
 X-COORD:

 DRILLED BY:
 DATE:
 Y-COORD:

 PROFILED BY :
 DATE :

 TYPE SET BY :
 DATE : 05/02/2018 15:42

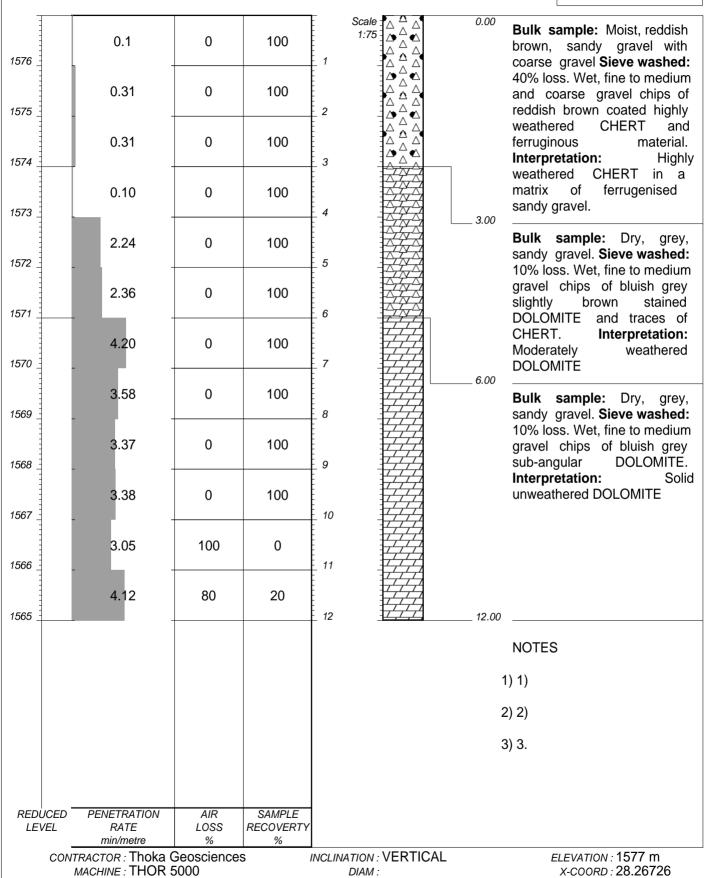
SETUP FILE: STANDARD.SET TEXT: ...\ProfileLogs\ESNBH11.TXT

LEGEND SUMMARY OF SYMBOLS



HOLE No: 12 Sheet 1 of 1

JOB NUMBER: ESN BH 12



DATE .

DATE: 12 JANUARY 2018

TEXT: ..\ProfileLogs\ESNBH12.TXT

DATE: 05/02/2018 15:46

PROFILED BY: Melusi Mashego

DRILLED BY:

TYPE SET BY:

Y-COORD: 26.00656

HOLE No: 12

Esselen Substation, Tembisa



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 12

/ / / / / / / / / / / /	DOLOMITE	{SA13}
	CHERT	{SA21}
	FERRICRETE	{SA24}

CONTRACTOR:INCLINATION:ELEVATION:MACHINE:DIAM:X-COORD:DRILLED BY:DATE:Y-COORD:

 PROFILED BY :
 DATE :

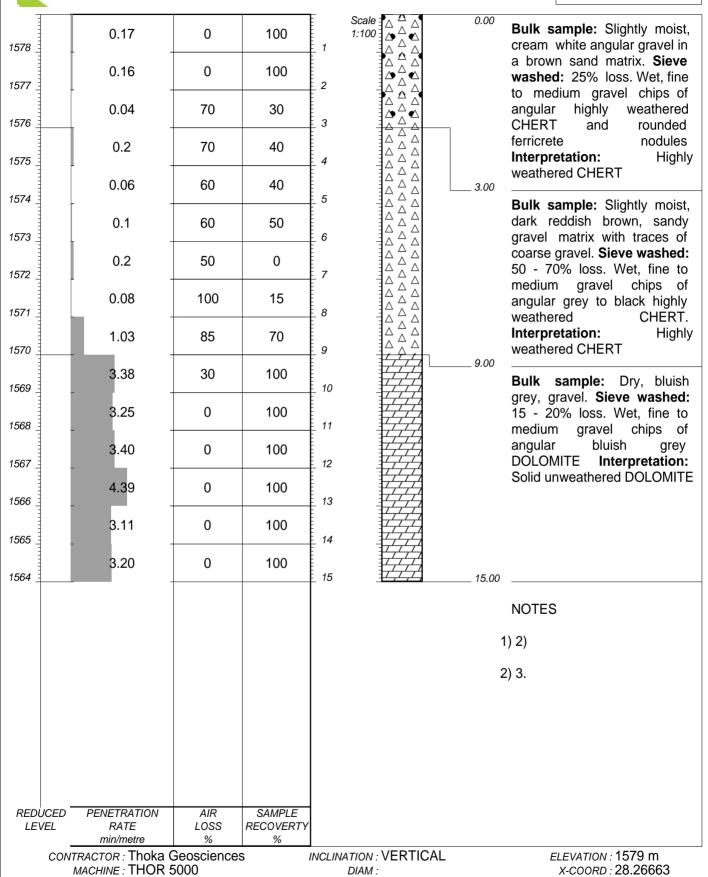
 TYPE SET BY :
 DATE : 05/02/2018 15:46

SETUP FILE: STANDARD.SET TEXT: ...\ProfileLogs\ESNBH12.TXT

LEGENDSUMMARY OF SYMBOLS

HOLE No: 13 Sheet 1 of 1

JOB NUMBER: ESN BH 13



DIAM:

DATE:

DATE: 10 JANUARY 2018

TEXT: ..\ProfileLogs\ESNBH13.TXT

DATE: 20/03/2018 14:57

DOAA Thoka GeoSciences

PROFILED BY: Melusi Mashego

SETUP FILE: STANDARD.SET

DRILLED BY:

TYPE SET BY:

dotPLOT 7016 PBpH67

Y-COORD: 26.00495

HOLE No: 13

Esselen Substation, Tembisa



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 13

/ / / / / / / / / / / /	DOLOMITE	{SA13}
	CHERT	{SA21}
	FERRICRETE NODULES	{SA24}

CONTRACTOR:INCLINATION:ELEVATION:MACHINE:DIAM:X-COORD:DRILLED BY:DATE:Y-COORD:

TYPE SET BY: DATE: 20/03/2018 14:57

PROFILED BY:

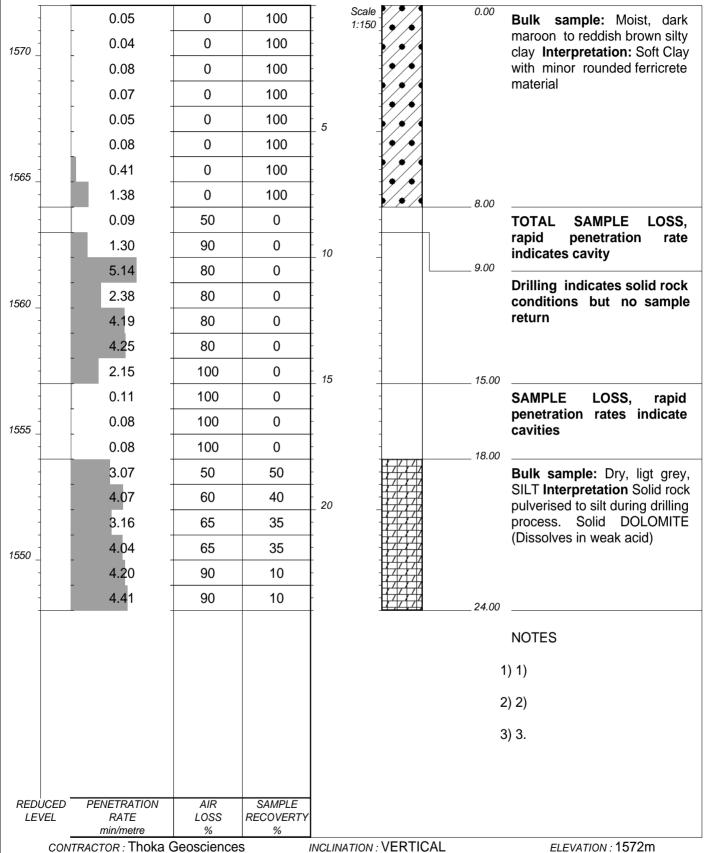
SETUP FILE: STANDARD.SET TEXT: ..\ProfileLogs\ESNBH13.TXT

LEGENDSUMMARY OF SYMBOLS

DATE:

HOLE No: 15 Sheet 1 of 1

JOB NUMBER: ESN BH 15



MACHINE: THOR 5000

DRILLED BY:

PROFILED BY: Melusi Mashego

TYPE SET BY:

SETUP FILE: STANDARD.SET

DIAM:

DATE: DATE: 17 JANUARY 2018

DATE: 30/01/2018 07:55

TEXT: ...\ProfileLogs\ESNBH15.TXT

X-COORD: 28.27112 Y-COORD: 26.00410

HOLE No: 15 Esselen Substation, Tembisa



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 15

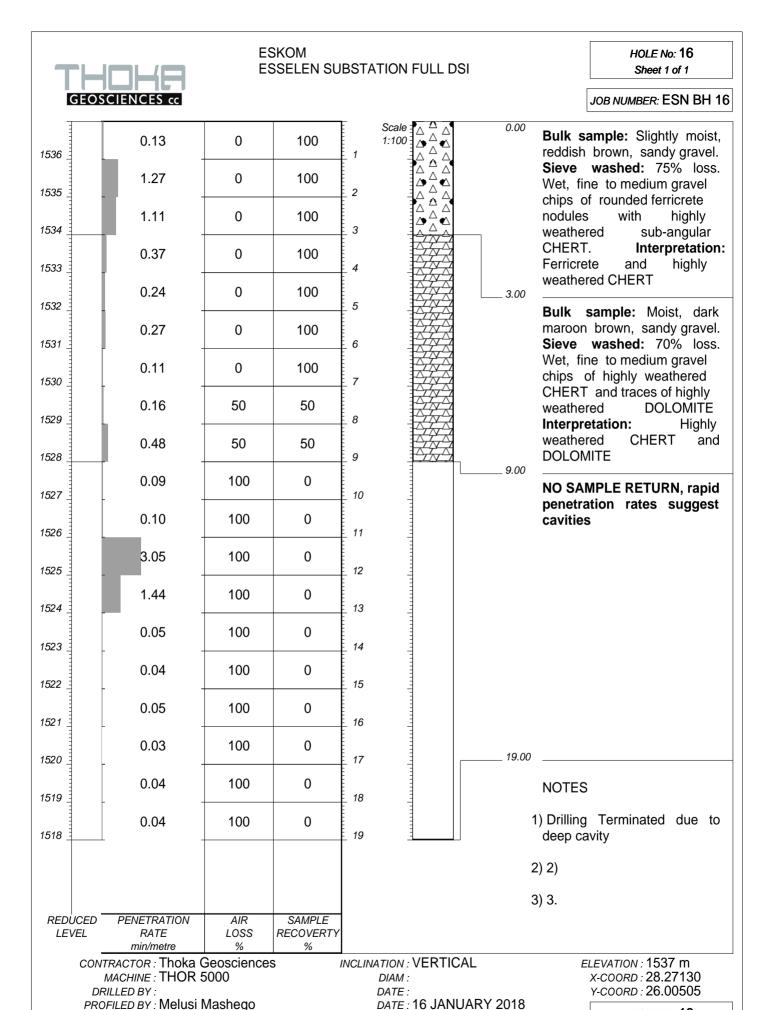
	SILT	{SA06}
	CLAY	{SA08}
	DOLOMITE	{SA13}
•	FERRICRETE	{SA24}

CONTRACTOR:INCLINATION:ELEVATION:MACHINE:DIAM:X-COORD:DRILLED BY:DATE:Y-COORD:

PROFILED BY: DATE:

TYPE SET BY: DATE: 30/01/2018 07:55
SETUP FILE: STANDARD.SET TEXT: ..\ProfileLogs\ESNBH15.TXT

LEGENDSUMMARY OF SYMBOLS



DATE: 01/02/2018 11:04

TEXT: ..\ProfileLogs\ESNBH16.TXT

SETUP FILE: STANDARD.SET

DOAA Thoka GeoSciences

TYPE SET BY:

dotPLOT 7016 PBpH67

HOLE No: 16

Esselen Substation, Tembisa



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 16

/ / / / / / / / / / / /	DOLOMITE	{SA13}
	CHERT	{SA21}
	FERRICRETE NODULES	{SA24}

CONTRACTOR: INCLINATION: ELEVATION:

MACHINE: DIAM: X-COORD:

DRILLED BY: DATE: Y-COORD:

DATE:

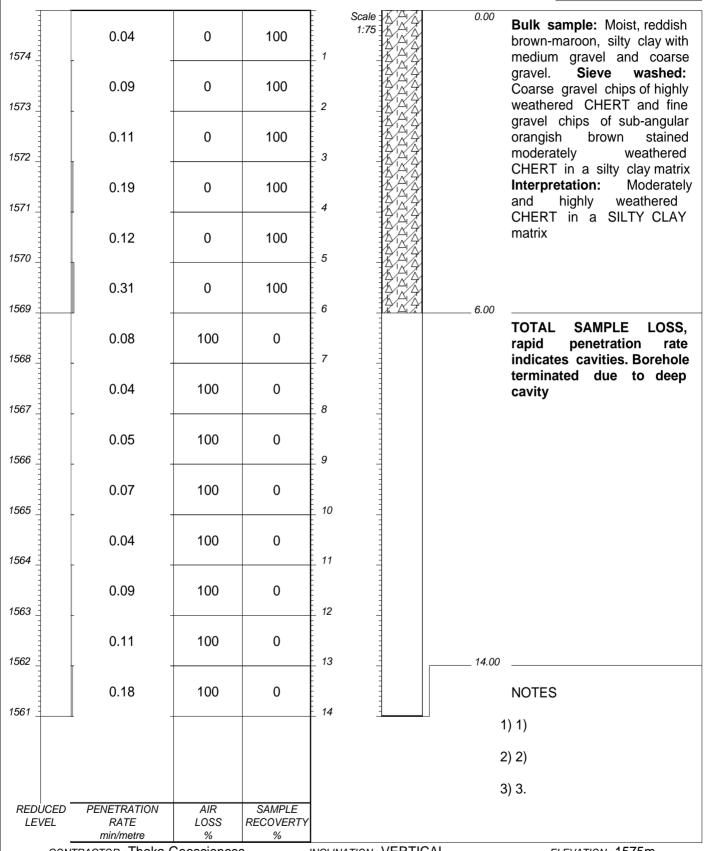
TYPE SET BY: DATE: 01/02/2018 11:04

SETUP FILE: STANDARD.SET TEXT: ..\ProfileLogs\ESNBH16.TXT

LEGENDSUMMARY OF SYMBOLS

HOLE No: 17 Sheet 1 of 1

JOB NUMBER: ESN BH 17



CONTRACTOR: Thoka Geosciences

MACHINE: THOR 5000

DRILLED BY:

PROFILED BY: Melusi Mashego

TYPE SET BY:

SETUP FILE: STANDARD.SET

INCLINATION: VERTICAL

DIAM: DATE:

DATE: 17 JANUARY 2018

DATE: 26/01/2018 13:41

TEXT: ..\ProfileLogs\ESNBH17.TXT

ELEVATION: 1575m X-COORD: 28.270016 Y-COORD: 26.00357

> HOLE No: 17 Esselen Substation, Tembisa



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 17

 SILTY	{SA07}
CLAY	{SA08}
CHERT	{SA21}

CONTRACTOR:INCLINATION:ELEVATION:MACHINE:DIAM:X-COORD:DRILLED BY:DATE:Y-COORD:

DATE:

TYPE SET BY: DATE: 26/01/2018 13:41

SETUP FILE: STANDARD.SET TEXT: ..\ProfileLogs\ESNBH17.TXT

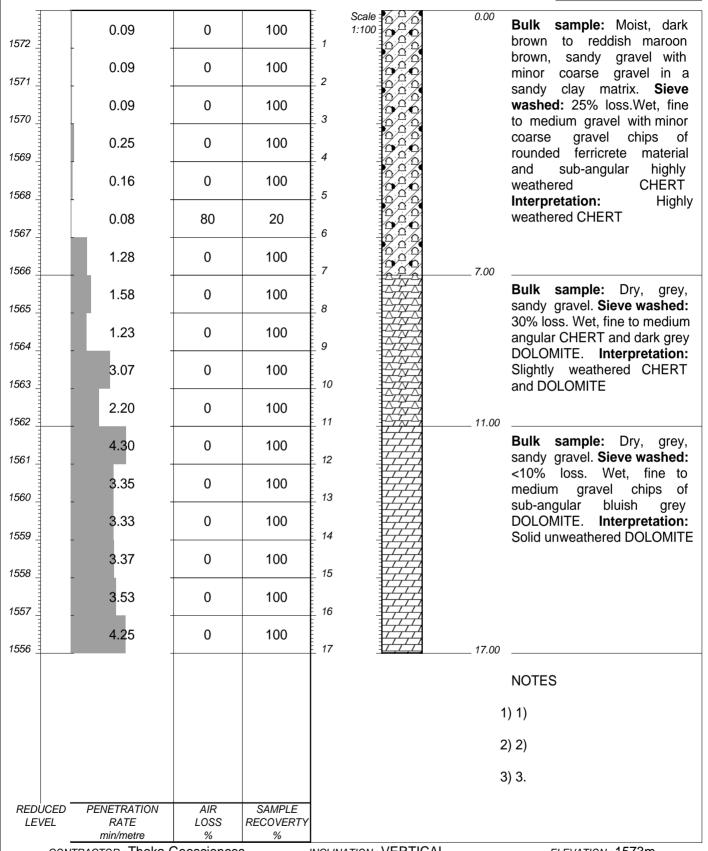
LEGENDSUMMARY OF SYMBOLS

THOHE GEOSCIENCES CO

ESKOM ESSELEN SUBSTATION FULL DSI

HOLE No: 18 Sheet 1 of 1

JOB NUMBER: ESN BH 18



CONTRACTOR: Thoka Geosciences

MACHINE: THOR 5000

DRILLED BY:

PROFILED BY: Melusi Mashego

TYPE SET BY:

SETUP FILE: STANDARD.SET

INCLINATION: VERTICAL

DIAM : DATE :

DATE: 15 JANUARY 2018

DATE: 01/02/2018 15:07

TEXT: ..\ProfileLogs\ESNBH18.TXT

ELEVATION: 1573m X-COORD: 28.26968 Y-COORD: 26.00622

HOLE No: 18
Esselen Substation, Tembisa



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 18

000	GRAVEL	{SA02}
	SANDY	{SA05}
	CLAY	{SA08}
	DOLOMITE	{SA13}
	CHERT	{SA21}
	FERRICRETE	{SA24}

DATE:

TYPE SET BY: DATE: 01/02/2018 15:07

SETUP FILE: STANDARD.SET TEXT: ..\ProfileLogs\ESNBH18.TXT

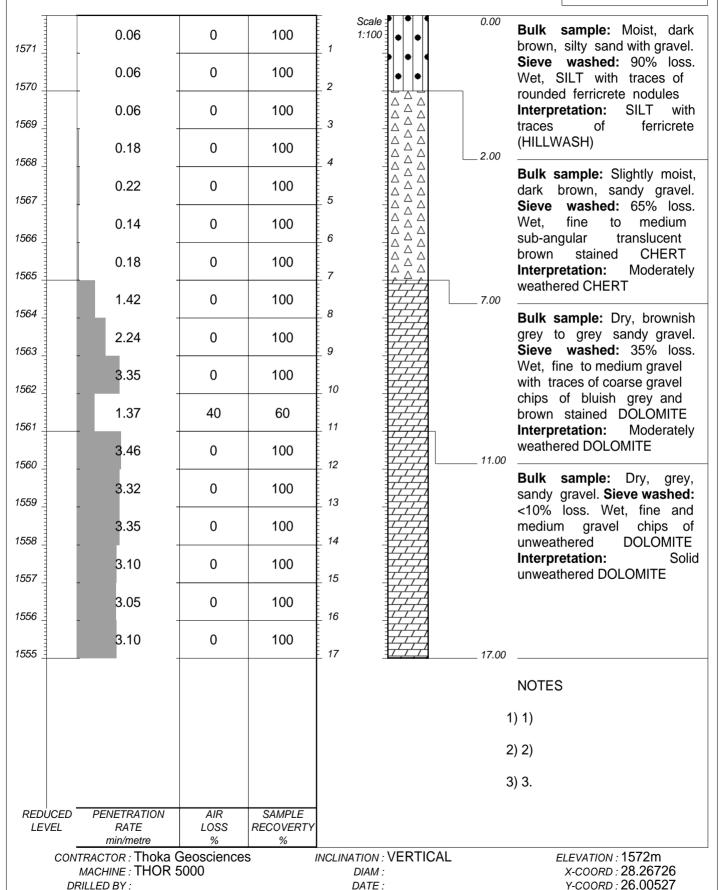
LEGEND SUMMARY OF SYMBOLS

THOHA GEOSCIENCES CO

ESKOM ESSELEN SUBSTATION FULL DSI

HOLE No: 19 Sheet 1 of 1

JOB NUMBER: ESN BH 19



DATE: 12 JANUARY 2018

TEXT: ..\ProfileLogs\ESNBH19.TXT

DATE: 31/01/2018 11:35

TYPE SET BY:

PROFILED BY: Melusi Mashego

SETUP FILE: STANDARD.SET

HOLE No: 19

Esselen Substation, Tembisa



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 19

	SILT	{SA06}
7 7 7	DOLOMITE	{SA13}
	CHERT	{SA21}
• •	FERRICRETE NODULES	{SA24}

CONTRACTOR:INCLINATION:ELEVATION:MACHINE:DIAM:X-COORD:DRILLED BY:DATE:Y-COORD:

DATE:

TYPE SET BY: DATE: 31/01/2018 11:35

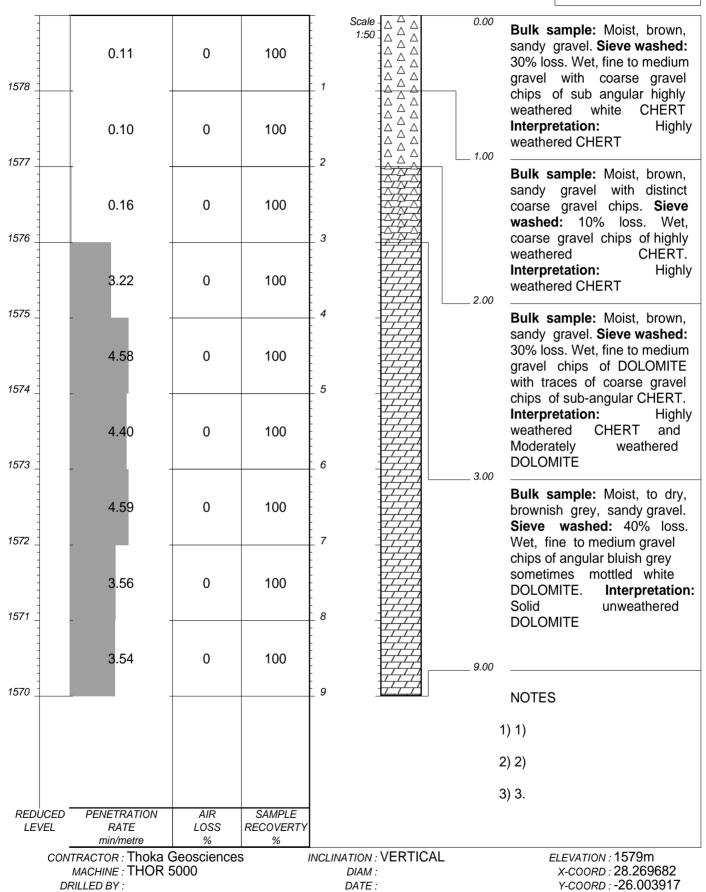
SETUP FILE: STANDARD.SET TEXT: ..\ProfileLogs\ESNBH19.TXT

LEGENDSUMMARY OF SYMBOLS



HOLE No: 20 Sheet 1 of 1

JOB NUMBER: ESN BH 20



DATE: 15 JANUARY 2018

TEXT: ..\ProfileLogs\ESNBH20.TXT

DATE: 01/02/2018 15:28

TYPE SET BY:

PROFILED BY: Melusi Mashego

SETUP FILE: STANDARD.SET

HOLE No: 20

Esselen Substation, Tembisa



LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 20

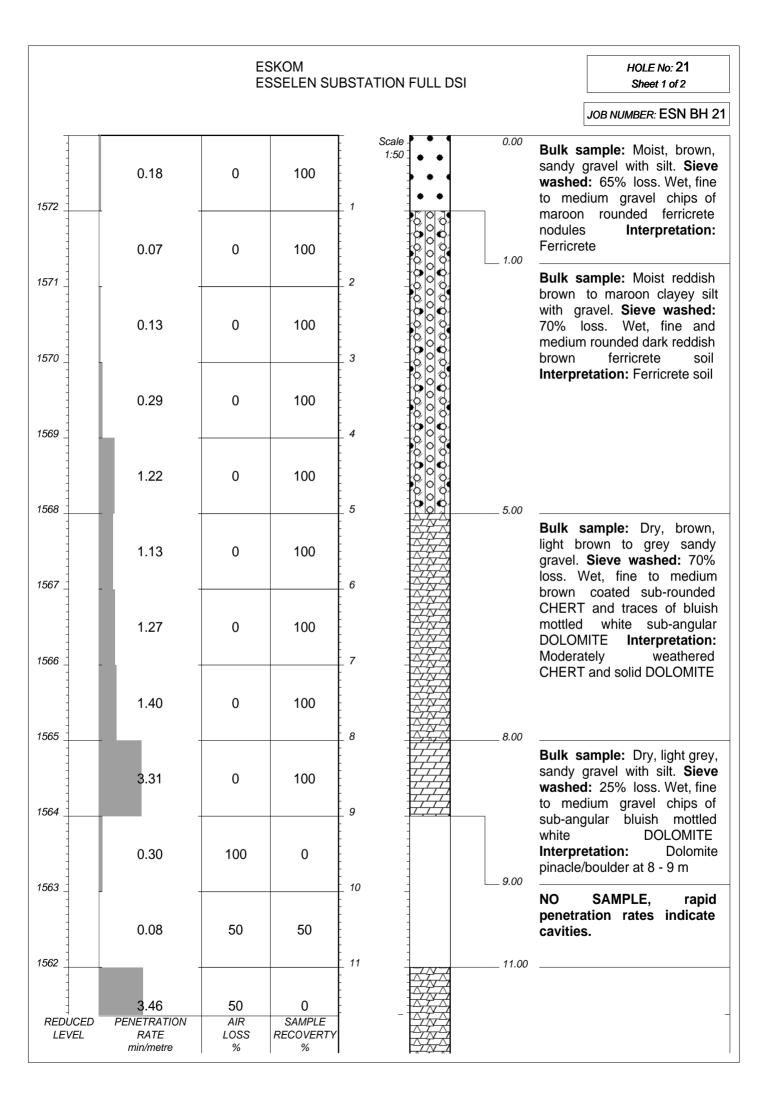
DOLOMITE	{SA13}
CHERT	{SA21}

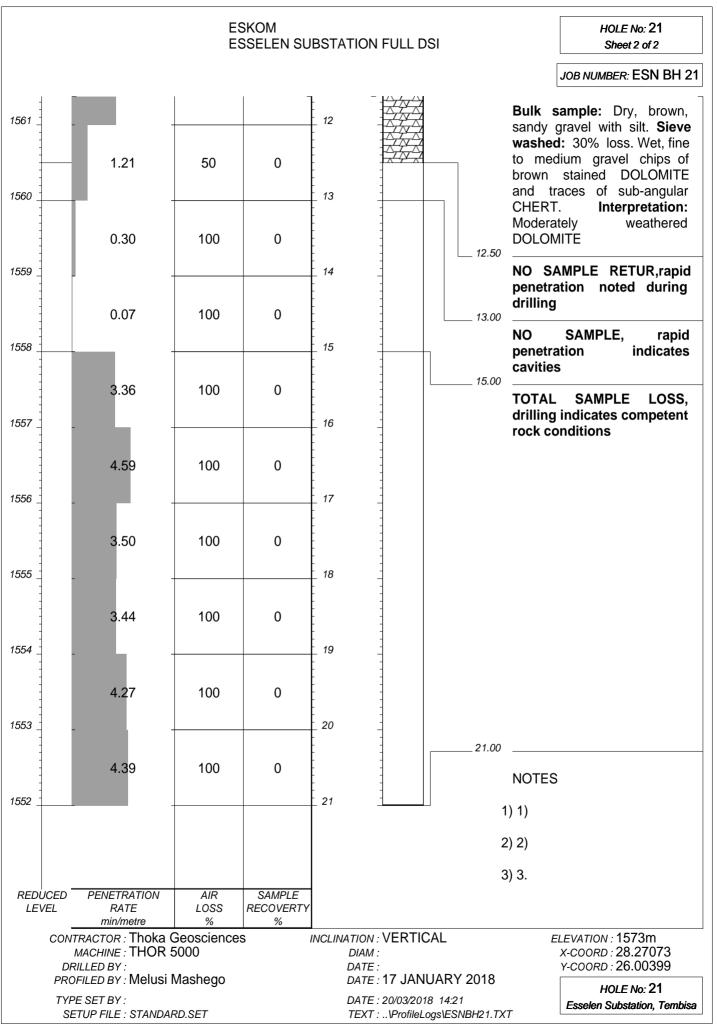
 PROFILED BY :
 DATE :

 TYPE SET BY :
 DATE : 01/02/2018 15:28

SETUP FILE: STANDARD.SET TEXT: ...\ProfileLogs\ESNBH20.TXT

LEGENDSUMMARY OF SYMBOLS





LEGEND Sheet 1 of 1

JOB NUMBER: ESN BH 21

000	GRAVEL	{SA02}
	SILT	{SA06}
	CLAYEY	{SA09}
	DOLOMITE	{SA13}
	CHERT	{SA21}
• •	FERRICRETE NODULES	{SA24}

CONTRACTOR: INCLINATION: ELEVATION: MACHINE: DIAM: X-COORD: DRILLED BY: DATE: Y-COORD:

DATE:

TYPE SET BY: DATE: 20/03/2018 14:21

SETUP FILE: STANDARD.SET TEXT: ..\ProfileLogs\ESNBH21.TXT

LEGEND SUMMARY OF SYMBOLS

Appendix IV

Drillers Logs



Head Office: 35 Woltemade Street, Culemborg Park, Randfontein, 1759, Postal Address: P O Box 1910, Silverton, 0127 Tel/Cell: +27 82 932 8465 Fax: +27 86 516 8363 Email: thokageo@gmail.com

PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION		
Client: ESKon	BH Name: ESN BH WL	Date: / 1/2018	Air Capacity (m3/m):	
Project: Apploan To STUD	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):	
Location: EISELEW -	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):	
(Coord: 1) 6,00 11	Elevation (m):	Water Rest Level (m) (After 24 Hrs):		
Coord: 7 12 6710	Bit Diameter (mm):	Water Strike (m):		
General Remarks:				

Depth	Penetration Rate (min/m)	Formation Hardness	Hammer Rate	Air Loss	Sample Recovery	Moisture Condition	Hole Collapse	Water Strike	Water Foam Applied	Comments
0-1	0.07	Soft	Roquiew	090	100%	S. moo	k			
1-2	0.07	19	d	1)	11	11				
-3	10.07	11	11	10	II	12				111
-4	0-32	15	- 11	ec	.,	er				1 Blest
-5	0.00	1	1 reporter	70%	30%	7				tasta
-6	12.05	CAVIT		11	11	(1)				d
5-7	0.05	Coson	1	4						Clark
7-8	0.05	<u> </u>		100%	0%	17				400
3-9	0 0 1	-	Regular	201	100%	CABARN				100
	2.25	-		0/0	10010	SIAMON			-	
-10			le!	1/	11	11				N D
0-11	1.20		1/	11	11	11			113	
1-12	2035		11	11	11	11				00/
2-13	4.12		11	- ((11	11				10-01
3-14	3.40		(1	11	F #	BRY				
4-15	3.59		15	11	11	116				
5-16	2.40	·	1.1	/1	11	56			- A	
6-17	3:56		11	15	11	15			5	
7-18	4.00		1	11	/1	-11		4		
18-19	/	2 0	v.			_				
19-20		-	31							
20-21					4					
21-22			1 1							
22-23										
23-24	-									
24-25										_
5-26								 		
6-27										
27-28	7 7 7	77						-		
28-29								+		
9-30		F-1			+					
30-31										
		-								
31-32										
32-33										•
33-34										
84-35										
35-36										
36-37										
7-38										
8-39										
9-40										
0-41										
1-42										
2-43			3/1	16.7				7		
3-44										
4-45										
5-46										
6-47		4-								-9.
7-48							MIN			
8-49								1		
9-50										
0-51										
1-52										
2-53										
3-54									-	
1-55					 					
5-56					 					1012/4/20
										
5-57					-					10.0.400
7-58				the state of the s	-					111111111111111111111111111111111111111
3-59										
9-60			hon ga						A ARLI	

Rig Operator: PRINCE MONSOS
Data Descriptions by: JSELIN CEPAJA

Doc Engine Compressor Start: Start:

Finish: Finish: FIELD ASSISTANTS
BONGAN, SABEBE
BELLO MIGWENXE
LUFOFE MAPUMPARM
HULAN, MALLEICE

THOKA GEOSCIENCES CC



Head Office: 35 Woltemade Street, Culemborg Park, Randfontein, 1759. Postal Address: P.O. Box 1910, Silverton, 0127 Tel/Cell: +27 82 932 8465 Fax: +27 86 516 8363. Email: thokageo@gmail.com

PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION	
Client: Eskou	BH Name: ESN BLH DA	10	
	1201	Date: 09 101 2018	Air Capacity (m3/m):
Project: AULOWITE STUDY	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):
Location: Esselew	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):
X Coord: 26.00 459°	Elevation (m): 1581 44	Water Rest Level (m) (After 24 Hrs):	, , , , , , , , , , , , , , , , , , , ,
Y Coord: 29- 266380	Bit Diameter (mm):	Water Strike (m):	
General Remarks:			

Depth	Penetration Rate (min/m)	Formation Hardness	Hammer Rate	Air Loss	Sample Recovery	Moisture Condition	Hole Collapse	Water Strike	Water Foam Applied	Comments
0-1	0.10	-	Regular	090	100%	S.Nois	4		7.	
1-2	0.11		6	li	11	11	-		,	1-16-
2-3	0.12		11	9	()	ef				Colberius
3-4	0.17		tj	12	10	19				
4-5	0.49		er	11	11	tr			-	The state of the s
5-6	0.49		11	1	, (11				111-10-
6-7	0.36		60%	60%	40%	15		-		(TEX)
7-8	0.08	21-	Ma	090	100%	- 1/	A	<i>j</i>		
8-9	0.06		-	11/0	100 10		VI	-		
9-10	0-06			1	Af .		1			7 7 7
10-11	0-06	(AUIT	4	fr	()		X			
11-12	0-06			٢/	11					
12-13	0.06	1/		11	15					
13-14	0.06	11		Ú	rf.					
14-15	0-06	00		c,	.,	100				
15-16	0.06	1//		d	1					
16-17	0.06	7		U	ci			A	B.	nned It
17-18	0-06	10		U	11			1	HUND	ONEO IT
18-19	0.06	1/		/				-	1 1	A
19-20	1							A	POLE	BUE TO
20-21		, <u>, , , , , , , , , , , , , , , , , , </u>						()	p .	
21-22								6	EDP	01
2-23										(411110)
3-24										
4-25									_	/
5-26										
6-27										
7-28										
8-29	STATE OF THE PARTY				-/					
9-30		-								
0-31	<u> </u>									
1-32										
2-33										10 10 10 10 10 10 10 10 10
3-34					-					
4-35										
5-36										
6-37										
7-38										
8-39 9-40										
0-41										
1-42									:-	
2-43										
3-44										
4-45										
5-46									-	
6-47							7			
7-48							х о			
8-49	×									
9-50										
0-51										
L-52	1	4								
2-53		- y								
3-54		- 62								
1-55										
5-56				1						
5-57		1								
7-58		4 2								
3-59										
-60		5 MON								

Rig Operator: PRINCE MOSSIVE Data Descriptions by: TSELISO LEBMA

Doc Engine Compressor

Start:

Finish: Finish: FIELD ASSISTANTS;

SELLO MGWENYE BONGAN, GADEBE Lufefe MPAKUMPAKY Hlulan, MALIGIE

THOKA GEOSCIENCES CC

The boxeholes has collapse to soonm



Head Office: 35 Woltemade Street, Culemborg Park, Randfontein, 1759. Postal Address: P O Box 1910, Silverton, 0127 Tel/Cell: +27 82 932 8465 Fax: +27 86 516 8363 Email: thokageo@gmail.com

PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION	
Client: ESkow	BH Name: ESN. BH 03	Date: 09/01/2018	Air Capacity (m3/m):
Project: DOCOMITE STUDY	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):
Location: Esseleas	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):
X Coord: 26.00 5 31	Elevation (m): 15 74 m	Water Rest Level (m) (After 24 Hrs):	
Y Coord: 28.265-90	Bit Diameter (mm):	Water Strike (m):	the same of the sa
General Remarks:			

Depth	Penetration Rate (min/m)	Formation Hardness	Hammer Rate	Air Loss	Sample Recovery	Moisture Condition	Hole Collapse	Water Strike	Water Foam Applied	Comments	
-1	0.18		1			Sin	remove				-
-2	0.20		1		1	11	Minne			CHERT	u
-3	0.16					11					1
4	6.33					10	-			7	1
-5	0.33			090	10090	11	-			1 FIRT	01
-6	0-40			111		15					1
-7	0.47		N	200%	40%	(1					1
-8	69.03	CAVITY		10090	10%	(i					1
-9	031		1/\	10076	4014	1,	replane			0 /	1
-10	0.08	4	1/	10090	40%	11	-			1//	1
0-11	0.04	CAVITY		Company Co	4090	11				1//	1
1-12	0.38		1 Lyneber	7040	3090	()				1//////////////////////////////////////	1
2-13	10-14		0	30%	35%	Sin	_			1 Charles	ł
3-14	0.08	1.4.	1 4	1000%	690	- 11	_			/	ł
1-15	0-08	AUN	y .	11	(1	14	-			,	1
5-16	0-08		/	11	E' B.	3 1	-				1
5-17	0-29	-	Rogulow	60%	4-010	11	~				1
7-18	2.55	Hourd	111	70000	3090	11	/				-
3-19	3.10	11	f	50%	5000	1/4					
9-20	3.44	, (71	0%	100%	NRY		parameter to the same		4	Spinster
0-21	3.55	. 0		25	115	11					
1-22	3:49	()	i «	11	, , ,	10				0	
2-23	3.58	(1)	11	7.1	11		1				
3-24	4.110	11	A r	11	11	11				1 PS ON	177
-25	7	1	`		- '	-//	4 -			100m	110
-26						1 1				() = + +	
5-27							1				
-28						1,01	-1				
1-29							1				
-30											
-31							11.				
L-32						I					
2-33											
1-34										h	
-35											
-36						140					
-37											
-38							1				
-39	- The		fi fi								
-40									1		
-41	1.54					l is					
-42					1 11 1						
-43			11 - 11	1,21							
44		_				V III					
45			5 E								
46		100		1							
47				y Y		4					
48		*	H	2 . I		100				_ 10	
49			1 2								
50		-			- 100	1				, ,	
51								7.71	The first of	Trock =	
52			Ary I		1904-1213				14 /		
53	(#)		e e e e e e e e e e e e e e e e e e e	- 1	HAMM P			-	1.2		
54											
55											
56	N. Comment		4 25							-	
57			1				710		x		
58	h ×		A					171 - 171			
59	1 4				17.				111		
50					3000	111	11				

Rig Operator: Phinte Mon Swe Data Descriptions by: TSENSO (EBA) A

8 26.00531 é 28-26590

Data Descriptions by: 1 2 4 4

Doc Engine Start:
Compressor Start:

FIELLO ASIN STANTS:

Sallo night Nyé

EN BANT SAPANIMANCES CO

Life to a paren marin

Hu Cam malu bira



Head Office: 35 Woltemade Street, Culemborg Park, Randfontein, 1759, Postal Address: P O Box 1910, Silverton, 0127 Tel/Cell: +27 82 932 8465 Fax: +27 86 516 8363 Email: thokageo@gmail.com

PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION	
Client: ESFO	BH Name: ESN BH 3	Date: 09 / 0/ 20 / 8	Air Capacity (m3/m):
Project: golowite staby	Total Depth (m):	Air Pressure (KPa):	Casing In & Out (m):
Location: Effection	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):
X Coord: 26.00 5310	Elevation (m): 1574~	Water Rest Level (m) (After 24 Hrs):	
Y Coord: 28 - 265 90 0	Bit Diameter (mm):	Water Strike (m):	
General Remarks:			

Depth	Penetration Rate (min/m)	Formation Hardness	Hammer Rate	Air Loss	Sample Recovery	Moisture Condition	Hole Collapse	Water Strike	Water Foam Applied	Comments	
0-1	0118			, ,		5. moi	+			1.6	1
1-2	0-20					11	/			1 alectrus	
2-3	0-16					11				Court	1
3-4	0-33										-
4-5	0-33			0%	100%	15				001	01
5-6	0.40			11	n					CHECTA	/ las
6-7	0.47			.01	114	(1		+		7	cary
7-8	0.08	(AVITI	/	6096	25/19	- (1					
8-9	0.31	CAVII		00 70	/	11				21	
9-10	0.08			1)	11	"		 	,		
10-11	0.03	CAVITI	,	17	11	4					
11-12	0.38	70//	1 ARegular	v //	11	Y				C CALL	
12-13	0.14		1 me pur	110		1/					
13-14	0.08		- 1	100%	46%	71					
14-15	0.08	(AVIT	1/	19070	11						
15-16	0.08	HUI	1	6000	40%	1/		-			_
16-17	0.29		1200.100	70%	3016	11					
17-18	2.55	Hard	Regulon	70/0	30/6	1)					
	3.10	11		50%	1200			-			
18-19 19-20	3.44	//	1)	0%	1 0-9/	DRY				1	
20-21	3-55	11		7	100/0	DAY					
	3-169	//		- 11							
21-22	3.49	11	5/	17	()	11				60/0m	17
22-23	4.10			- !/ .	- 11) /				100000	116
23-24	4.10	, ,	1/		•/	'/					
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50-51							,	1			
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52-53								-			
53-54	However the second								W		
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55-56	15015										
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8-59						3.					
9-60		a March	0 . 6						a =0 a=	A	

Rig Operator: PRINTE MOY SWE DATA Descriptions by: TELESO EDASA

Doc Engine Compressor Start: Start: Finish: Finish:

THOKA GEOSCIENCES CC

24m no collapse borehole its day FICES AFFESTANTS.
SELLO ANSWENYE
LENGO FE MARCHIMPARIS
BONGANI SASOBE
HULANI MALLO OS



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PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION		
Client: ESLOW	BH Name: BSN BH	Date: 15/01/2018	Air Capacity (m3/m):	
Project: ANDWITE STABIL	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):	
Location:	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):	
X Coord: 16,00/26	Elevation (m):	Water Rest Level (m) (After 24 Hrs):		
Y Coord: 98 261113	Bit Diameter (mm):	Water Strike (m):		
Y Coord: 28 25 143	Bit Diameter (mm):	Water Strike (m):		

3-7 3-9 3-10 0-11 1-12	0.15 0.34 0.65 0.05 0.05 0.05 0.05 0.05 0.05	Carry	Ropelor 11 14 15 repelor 17 17 17 17 17 17 17 17 17 1	60%	100/3 (1 (1 40% (1	5.000				askers
-2 -3 -4 -4 -5 -6 -7 -8 -9 -10 0-11 11-12 2-13 3-14	0.34	COSTY	rerepely	60%	(1 46% (1 140%	11				
-3 -4 -4 -5-5 -6-6 -6-7 -7 -8-8 -9 -10 -0-11 -11-12 -2-13 -1-14	0.65 0.65 0.05 0.05 0.05 0.05 0.05 0.05	COSTY	rereased Property	60%	(1 46% (1 105%	11				1
-4 -4 -5 -6 -6 -7 -7 -7 -7 -7 -7 -7 -1 -1 -1 -1 -1 -1 -1 -1 -1 -1 -1 -1 -1	0.65	COSTY	Peregular Property	60%	46% (1 (0)/6	11				100
-5 -6 -7 -8 -9 -10 00-11 11-12 22-13 33-14	0.05 0.05 0.05 0.05 0.05 0.05	COSTY	POGO	10%	100/6					Chert &
6-6 6-7 7-8 8-9 9-10 0-11 1-12 2-13 3-14	0-05" 0-05" 0-05" 0-05" 0-05"	COSTY	11	190%	100%					4
-7 -8 -9 -10 0-11 1-12 2-13	0.05" 0.05" 0.05" 0.05"		11	- (1	(HO)	15	(Manager Land)			WIAM
7-8 3-9 3-10 0-11 1-12 2-13 3-14	0.05				41	Ser.				1000
1-9 1-10 0-11 1-12 2-13 3-14	0.05			1/	1.	7				1
0-10 0-11 1-12 2-13 3-14	0.05			25	cc	/1		0.	. /	
1-12 / 2-13 (3-14	9.05		er	U	(1	A125-	181	Vo San	DI-	
2-13	-		C/	7.	11	1011	79	GA!	1/2	
3-14	0-05		()	17	11	/				
			1's	18	1,	1				
1.15										
1-13										11/1
5-16										HOLE ABI
6-17										
7-18										No 1
8-19										will do
9-20										7
0-21										la at
1-22										Course
2-23		\	6							0
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Rig Operator: TS=450 LEGASA

Doc Engine Compressor Start: Start: Finish:

THOKA GEOSCIENCES CC

FIELD ASSISTANTS:

benson, GADERE

SECIO MENONYE

LUFO FO MISSU UPARA

HIGHNI MALL LERO

5 26.00600° E28.76692° ECG = 15 FT -Version Date: May 2015



Percussion Borehole Drilling Records

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PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION		
Client: 55 Kory	BH Name: ESN BH	Date: 12/01/2018	Air Capacity (m3/m):	
Project: Dolo-176 174 A	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):	
Location: EUELEN	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):	
X Coord: 26 00600	Elevation (m): 15 75 cm	Water Rest Level (m) (After 24 Hrs):		
Y Coord: 28.26692	Bit Diameter (mm):	Water Strike (m):		
General Remarks:				

Penetration Rate (min/m) Sample Recovery Moisture Condition Hole Collapse Water Foam Applied Air Loss SOFL Regulok 3.may 01 1008 000 0:24 1-2 11 100 11 DRY 806m,72 11 2-3 22 100 11 3:01 2 11 HARD 11 100 M 11 11 11 11 7.1 X 11 11 11 7-8 11 11 17 11 8-9 11 11 11 11 9-10 11 11 1 11 10-11 11-12 12-13 13-14 14-15 15-16 16-17 17-18 18-19 19-20 20-21 21-22 22-23 23-24 24-25 25-26 26-27 27-28 28-29 29-30 30-31 31-32 32-33 33-34 34-35 35-36 36-37 37-38 38-39 39-40 40-41 41-42 42-43 43-44 44-45 45-46 46-47 48-49 49-50 50-51 52-53 53-54 54-55 55-56 56-57 57-58 58-59 59-60

Rig Operator: PRINCE WON SW C Data Descriptions by: TSELIFE LEBASA

Doc Engine

Start:

Finish:

FIELD ASWISTBONTS: SELLO MGWENYE LUFEFE MPAKUM PAKU BONSANI GABEE HELANI MALI LOKE

THOKA GEOSCIENCES CC



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PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION		
Client: ESKom	BH Name: ESN BH OF	Date: /5/01/20/8	Air Capacity (m3/m):	
Project: polomen oraly	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):	
Location: ESELEN	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):	
(Coord: 26,00682	Elevation (m):	Water Rest Level (m) (After 24 Hrs):		
1 Coord: 7 7 26894	Bit Diameter (mm):	Water Strike (m):		
General Remarks:				

Depth	Penetration Rate (min/m)	Formation Hardness	Hammer Rate	Air Loss	Sample Recovery	Moisture Condition	Hole Collapse	Water Strike	Water Foam Applied	Comments
0-1	0.08	Soft	Rogelow	19/10	100%	mosst				
1-2	0.08	1	10/00	11	(1	1/		_		
2-3	0.60	11	T/	41	11	11				CHERT B
3-4	0.08	71 ,		11	11	Shoot				C/Con/ B
4-5	3.47	Hard	0	4	r	bly				-
5-6	2.50	15	11			S. Mouth			4.4	7
6-7	3,90	e,		- [/	11	3.0000			- A	R
	13.32	1)	ti.	(1	1				-	I A
7-8			7/	^1	,	7.5			X	(0000)
8-9	3.33	11	1.	11	116	[1			X	
9-10	3.27					DRY				
10-11						/				
11-12										
12-13										
13-14										
14-15										
15-16										
16-17										
17-18										
18-19								-		
19-20						 		-		
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20-21			-		_					
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22-23										
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25-26										
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7-28										
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2-33										
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4-35								-		
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6-37										
7-38										3,
8-39			-							
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3-44										
4-45							22946			
5-46										
6-47										
7-48								1		
8-49										
9-50										
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					1					
1-52	-		-					-		
2-53										
3-54										
4-55									T I	
5-56										
5-57										
7-58										
3-59										

Rig Operator: PRINCE MOYS WE DATA

Doc Engine Compressor

Start: Start: Finish: Finish: FIELD ASSISTANT:
BENGAN, SABERE
SELLO MENEN PRICES
Lufefe MISEL PRICES
Hulswy Malulope

THOKA GEOSCIENCES CC

Percussion Borehole Drilling Records

Version Date: May 2015



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PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION) "
Client:	BH Name: SSN SHOF	Date: 16/6//20/8	Air Capacity (m3/m):
Project:	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):
Location:	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):
X Coord: 216,00 4550	Elevation (m): 1571111	Water Rest Level (m) (After 24 Hrs):	
Y Coord: 2381271480	Bit Diameter (mm):	Water Strike (m):	
General Remarks:			

Depth	Penetration Rate (min/m)	Formation Hardness	Hammer Rate	Air Loss	Sample Recovery	Moisture Condition	Hole Collapse	Water Strike	Water Foam Applied	Comments	
0-1	0:06	SOFE	Regulos	2	100	Moist			1	Real Tep Sel	Y
1-2	0 . 05	11	11	0	100	11					
2-3	0:11	11	11	0	100	11					1
3-4	0:09	11	11	0	100	11					1
4-5	0:14	11	11	0	100	11					1 .
5-6	0:36	91	11	0	100	11				Red clay	Soil
6-7	0:38	11	1/	0	100	11					
7-8	0:20	11	11	ne same ne same no sam	Le						1.
8-9	0 08	1/	//	no 3AMP	16					Small Cavi	161
9-10	0:14	11	11	no sam	ple						1
10-11	3:43	HARL	U	308	70%	#1				Brilling	LANGE
11-12	3:51	HARD	11	0	100	1/		,		DRITTING	LINEDI
12-13	3:16	MECHUM	11	0	100	11					
13-14	3:36		11	0		11					ļ
14-15	3 :45	MARIA	11	0	100	11					
15-16 16-17		HARD	10	0	100	11			1 1		
17-18	3:49	HARA	ei	0	100	11					-
18-19	8:05	HARD	1	0	100				1		,
19-20		THE VIEW	<u> </u>		100	1/					
20-21	7										
21-22											1
22-23											İ
23-24											
24-25											
25-26											
26-27		0									
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39-40									-		
10-41			-	1100							
11-42											E-1
12-43									80.00		
13-44									100		
14-45											
15-46									THE STATE OF		
16-47			V					-			
7-48											
8-49											
9-50									19		
0-51											
1-52			Ti .								
2-53						-					
3-54											
4-55									-		
5-56											
6-57								-			
7-58								-	-	1	
8-59						1					

Rig Operator: Data Descriptions by:

Doc Engine Compressor Start: Start:



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PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION			
Client:	BH Name: ESA BITOS	Date: 1710112018	Air Capacity (m3/m):		
Project:	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):		
Location:	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):		
X Coord: 26,00 3850	Elevation (m): 15 74 m	Water Rest Level (m) (After 24 Hrs):			
Y Coord: 18 27 0950	Bit Diameter (mm):	Water Strike (m):			
General Remarks:		***************************************			

Γ	T	T	T	Γ	1	<u> </u>				
Depth	Penetration Rate (min/m)	Formation Hardness	Hammer Rate	Air Loss	Sample Recovery	Moisture Condition	Hole Collapse	Water Strike	Water Foam Applied	Comments
								0.000**0000		3
0-1	0 :06	SOFE	Regulas	08	1008	MORST				Red TOP Soil
1-2	0:06	11	11	0	100	1/				
2-3	0:07	11	11	0	100	21				1.6
3-4	0 112	11	11	0	160	11				
4-5	0 111	11	11	0	100	4			· ·	
5-6	0:16	11	11	0	100	11				
6-7	0:24	1)	1)	0	100	91				
7-8	1:20	11	IRegulos	0	100	11				calliby
8-9	0:07	11	11	nosami	PLR.	-				
9-10	0:58	11	1/	50 %	602	noist		-		(483)
10-11	0114	11	11	no SA	mper		1	Λ.		
11-12	0:03	11	12	11	11	autorité.	/	11 1	//	1
12-13	1:37	11	REGULA	11	11	-	/	#/1/	110	1
13-14	0 :11	11	nco or a		11	-		100	101	
14-15	3:50	HARD	11	902	11				/	
15-16	3:33	HARD		106	60	moist			1	The state of the s
	3:19	HARD	; i	50	60					
16-17 .	3:54	HARD	11	40		11				-
17-18	3:54	HARD		20	80	11			-	
18-19	3:54 3:29 3:40	TAND	11	10	90	1)				
19-20	3:40	HARB	11	02	100	11			-	
20-21			-							
21-22			1.4							
22-23										
23-24									-	
24-25			1			-				
25-26								to Commence		
26-27								The state of the s		
27-28										
28-29										
29-30										
30-31										
31-32								4		
32-33			7							
33-34										
34-35	8:									
35-36										
36-37		r.	NO.							
37-38			1 dg		- ye					
38-39	7		175		X III					
39-40			17/19/19							
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42-43			£177							
43-44										
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45-46			36-3-3-3-3-4							
46-47										
47-48				-		4				
48-49										
49-50						- 0				
50-51										
51-52										
52-53										
53-54	F _ H* 5					V	¥.			- Table 1
54-55										
55-56		Ų								
56-57								11-12-12		
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58-59										

Rig Operator: Data Descriptions by:

Dee Facine

Doc Engine Compressor Start: Start:

Percussion Borehole Drilling Records

Version Date: May 2015



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PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION		
Client:	BH Name: ESN B 109	Date: 17-10112018	Air Capacity (m3/m):	
Project:	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):	
Location:	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):	
X Coord: 26,003540	Elevation (m): 1579 m	Water Rest Level (m) (After 24 Hrs):	, and gen (iii)	
Y Coord: 28, 26943	Bit Diameter (mm):	Water Strike (m):		
General Remarks:				

	remarko.										
Depth	Penetration Rate (min/m)	Formation Hardness	Hammer Rate	Air Loss	Sample Recovery	Moisture Condition	Hole Collapse	Water Strike	Water Foam Applied	Comments	
0.1	0:10	30F7	Requial	0	100	10019					
0-1 1-2		SOFT	11		100	Moist					
	0:26	SOFT	11	0	100	11					
-3	0 37	SOFI		8		11					
-4	21:03	Meelium			100	DRY			1		
-5	4:19	HARD	11	0	100	10			1	HALF Mel	
6	1:53	SOFT	11	0	100	moist				HALF Mell	4.0
7	0:08	80FT	no ham	mes	.0%					Calcidated	246
8	0:07	11	11 11		02					200	
9	3:01	HARD	Regular	508	50%	moist					
10	3:32	11	11	30	50	11					
-11	3:09	11	1/	50	50	11					
-12	2:09	11	11	50	50	4)		-			
-13	3 21	11	11	402	60%			-			
	3139	11	1/	OF CO D		11					
14	2137	17	//	308	79			-	-		
15											
16		-			-						
-17		-									
-18		-									
19											
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21											
22											
23											
24											
25						1		1			
26						1					
-27									-		
28									-		
29						 					
30						1					
-31 -32											
		1									
33						1					
34		L									
35							C wall				
36				J	· · · · · · · · · · · · · · · · · · ·						
37						1 1 00					
38		Equ. ed	1 1 1	maj)	1						
39						1	213				
40		# 1 = F		-144	200						
11											
12		X 145 1			d.				1		
3									Z-2	/ IV	
14			Value of	1	F		4. 1.71	1			
15							4 27	- CV			
16		3 1 7 2	7	7	v e		1				
7	9										
8		7 17	(7 7				7	
19											
0		7	7						-		
			-								
1											
2		14-25-						l l			
3						1					
4											
5									-		
6											
7											
8											
9								1			
0			2 7 7 7								
								1			

Rig Operator: Data Descriptions by:

Doc Engine Compressor Start: Start:



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PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION	
Client: ESKO M	Dun 15/11 19/11/11	10/01/00/00	u
	BH Name: ESW BH 11	Date: 12/0//2018	Air Capacity (m3/m):
Project: Apolow (TE STUDY	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):
ocation: ESSELEN	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):
(Coord: 26.00573"	Elevation (m): 15 +9 um	Water Rest Level (m) (After 24 Hrs):	, and a second
Y Coord: 28, 26736	Bit Diameter (mm):	Water Strike (m):	
General Remarks:			

Penetration Rate (min/m) Formation Hardness Sample Recovery Moisture Hole Collapse Air Loss Water Strike Water Foam 5,000) 0.26 1, 0. 25 tx 2.3 11 0.08 10090 CAVITY 11 50% 090 11 iregular 0.50 50% 11 08 CAUTTY X 3.30 Rogulow 0% 10000 DRY 15/ 11 11 9-10 11 15 er 10-11 11 11 rr 4 11-12 7.53 7.58 10 01 12-13 ,1 17 11 13-14 11 11 11 14-15 15-16 16-17 17-18 18-19 19-20 20-21 21-22 22-23 23-24 24-25 25-26 26-27 27-28 28-29 29-30 30-31 31-32 32-33 33-34 34-35 35-36 36-37 37-38 38-39 39-40 40-41 41-42 42-43 43-44 44-45 45-46 46-47 47-48 48-49 49-50 50-51 51-52 52-53 53-54 54-55 55-56 56-57 57-58 58-59 59-60

Rig Operator: PAINLE MONS WE Data Descriptions by: 15E 4180 GEBSA

Doc Engine Compressor

Start:

Finish: Finish: FIELD ASSISTANTS: SEZLO MENEWYS LUFETE MIAKU MIAKU BONGAN, GADOBE HLUCAN, MALULOKE

THOKA GEOSCIENCES CC



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PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION .		
Client: 25 Kon	BH Name: ESN BH 12	Date: /2/0//20/8	Air Capacity (m3/m):	-
Project: Do Lawy TE STUDY	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):	
Location: ESSELEW	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damag	ged (m):
X Coord: 26.00 686	Elevation (m): 1577 m	Water Rest Level (m) (After 24 Hrs):	2 0	
Y Coord: 28.26726	Bit Diameter (mm):	Water Strike (m):		
General Remarks:				

Depth	Penetration Rate (min/m)	Formation Hardness	Hammer Rate	Air Loss	Sample Recovery	Moisture Condition	Hole Collapse	Water Strike	Water Foam Applied	Comments
)-1	0.10	Sola	Reguler	0%	100%	Charitte				1-11-100
-2	0.31	Soft	Lugiden			S.mossy	-	-		CHEVE , Jen
2-3	0-31	11	c _l	17	11	17		_	-	Cherry de des
-4	0.10	6		11	()	17				CKC. CT J'W
-5	2,24	4	- //	- U	1					J. Committee
	2 12	Hord	1/	()	(1	ORY			-	
-6	4.20	11	11	11	11	11/				1 1
-7		(/	()	1/	11	11				1
-8	13,58	()	1/	11	5/	1)		(4		
-9	3.37	4	()	1	11	11				50/0 ca 172
-10	3.38	11	1/	11000	11	11				1/6
0-11	3.05	CAVITY	ivaregular	8090	20%	11				
1-12	4.12	Hard vay	regulor	8070	00%	11				
2-13			,							
3-14										
4-15										
5-16										
6-17										
7-18										1 1 1
8-19								- 12	S. Jangarkin v.	POR MICHIGANIC CONTRACTOR
9-20								at the state of th		
0-21										
1-22					_					
2-23							-	100,000		(1) and to design the second
3-24						40	1112	and and the same		
4-25							500			
5-26			× -				-			
5-27	=									
7-28										
8-29										
9-30				-				1.7		
0-31										
1-32										
2-33									-	
3-34			00							
4-35									-	
5-36					-					
5-37										
7-38								-	-	
3-39									-	
9-40										
)-41 -42		-				-				
						-				
-43									-	
3-44								-		27-186
1-45						-				THE STATE OF
-46										250000000000000000000000000000000000000
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-58										
-59		1	_							
-60								1		

Rig Operator:
Data Descriptions by: TSEUSE CEBISM

Doc Engine Compressor Start:

Finish:

THOKA GEOSCIENCES CC

JELLO ASSISTANTS.

BELLO MSWENYE

BONGANI GADEBE

LUFEFÉ MPAKU NYAKU

HULANI MALULERE



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PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION	
	BH Name: ESN 314 13	Date: 09/01/2018	Air Capacity (m3/m):
Project: Delow ITE STUBY	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):
Location: ESELE N	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):
X Coord: 26.00 495	Elevation (m): 1579	Water Rest Level (m) (After 24 Hrs):	
Y Coord: 28.266639	Bit Diameter (mm):	Water Strike (m):	
General Remarks:	•		

Depth	Penetration Rate (min/m)	Formation Hardness	Hammer Rate	Air Loss	Sample Recovery	Moisture Condition	Hole Collapse	Water Strike	Water Foam Applied	Comments	
0-1	0.17		Logulla	0%		5. most				1.	
1-2	0-16		151	11/		11				C41087	
2-3	0.04	CAVITY	CAUTY	70%	30%	Cr				1/02	
3-4	0.20	() V /	iRfagular	111	U	()				P.	
4-5	0.06	CNILL	1	60%	40	1	X				
5-6	0.10	CAUITY	AUITY	11	10	17	X			11/4	
6-7	0-20		Ilkeyeler	50%	5080	11				40	
7-8	0.08	CAVITY	2/	·		11					
8-9	1.03	,		85%	15%	50					
9-10	3.38		Regulow	3080	10%	BARY					
10-11	3.25		91	0%	100%	11/	-			*	este.
11-12	3,40		il	11	11	11				1/1	
12-13	3.40		11	11	11	11				500	•
13-14	3:11		, /	1/	()	11				, /	LITE
14-15	3.20		11		11	'/					
15-16										1.1	
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10-41										(inch	
11-42											
12-43											
13-44											
14-45										11	
15-46									5.6		
16-47											
17-48											
18-49											
9-50											
0-51											
2-53											
3-54								in the second			
4-55								SIA			
5-56											
6-57								-			4"
7-58											
8-59								 			

Rig Operator: PRING MONSINE
Data Descriptions by: TSELISO LEBASA

Doc Engine Compressor

ALC:

Start: Start: Finish: Finish:

Dry hole
No Collepse

FIELD ASSISTANTS:
Lufefe Makumpakan
BONSAN, SADEBE
BELLO MGUENYO
HULAN, MAKUKA



Percussion Borehole Drilling Records

EST BHIS

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PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION	
Client:	BH Name: ESN 131-1 15	Date: 7-10112018	Air Capacity (m3/m):
Project:	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):
Location:	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):
X Coord: 216.00 4100	Elevation (m): 1572m	Water Rest Level (m) (After 24 Hrs):	
Y Coord: 28.271120	Bit Diameter (mm):	Water Strike (m):	
General Remarks:	The state of the s		

				The state of the s							
Depth	Penetration Rate (min/m)	Formation Hardness	Hammer Rate	Air Loss	Sample Recovery	Moisture Condition	Hole Collapse	Water Strike	Water Foam Applied	Comments	
)-1	0:05	SOFT	Regular	08	100%	moist				Red TOP SO	4
1-2	0:04	11	11	. 0	100	11	all straight of the	er a 1854 - 17 - 1		NCG 181 8	14-
-3	0 08	11	11	0	100	11	and the same of				-
	and the second second second second	11	11								4
4	0:07		//	0	100	1)					
5	0:05	11	11	0	100	11					
6	0:08	11	1)	0	100	17				CLAY SOIL	
7	0:41	11	11	0	100	17		V.			1
8	1:38	11	11	0	100	U.		×			1
9	0:09	11	no humnes	50%	50%	66				cavity	1
10	1:30	£)	REAUKIS	90		Catalog				Carried States	-
-11	5:14	HARN	11	80	440	Name (1 0 10 110	DA E
	21:38	mediun	1	22					-	BRITING HA	KAD L
-12		MARIA	1()	80	Courte	responses					bu
-13	4:19	HARD	11	80	County)	-	CL JUSCE	15.		11/2/10/2017	
-14	4.25	HARLS	11	30	Spenish	-	September 11	11		A PARTY OF THE PAR	SA
15	a115	medium	1	00		1.0				(8.整排)。	1
16	@ ! !!	SOFt	11	1-11	1/1/	l A					1
17	0:08	10	1)		VIO	91	100	ly is		785 4. 7	1
-18	0108	1)	11			1					1
-19	3:07	HARA	Regulat	508	50%	bry		<u> </u>	 	100 m of 1	-
-20	4:03	HARB							-		-
	3:16	IAP	11	60	40	11				- 1	1
-21	3:16	HARD	11	68	35	11 .					1
-22	4:04	HARA	11	65	35	11		, /	N. A.	J	
23	4:20	MARN	11	90	10	1/				· ·	1
24	4:41	MARIS	//	90	10	1)	19				1
-25			, in	,				1		t	1
-26				-				<u> </u>			1
-27				190							-
-28		- 2									-
		- 1				-	3 1			9,5	1
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Rig Operator: Data Descriptions by:

Doc Engine Compressor

Start: Start:

Percussion Borehole Drilling Records

1 Version Date: May 2015



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PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION	
Client:	BH Name: ESN 33M/6	Date: 16 10 112018	Air Capacity (m3/m):
Project:	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):
Location:	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):
X Coord: 26.00505	Elevation (m): 1537 m	Water Rest Level (m) (After 24 Hrs):	
Y Coord: 22 8- 217 130	Bit Diameter (mm):	Water Strike (m):	
General Remarks:			

Depth	Penetration Rate (min/m)	Formation Hardness	Hammer Rate	Air Loss	Sample Recovery	Moisture Condition	Hole Collapse	Water Strike	Water Foam Applied	Comments
0-1	0113	8cf-6	Regiles	08	100 %	moist				700 0011
1-2	1 27	11	11	0	100	11				TOP SOIL
2-3	1:11	11	11	0	100	11		-		
3-4	0:37	11	Thegray	0	100				-	- 11 - 10
4-5	3:24	H	W SILVER	0	100	11				Small cavi Small cavi
5-6	0:27	11	la la	0	100					Small cavi
6-7	0:11	11	11	0	100	11				Small carri
		11	1/	0	100	11				cavi by
7-8	0 48		11	508	50	11				
8-9		11	/	500	50	1/				
9-10		11	19	100	0	11	-			eau, by
10-11	0:10	11	r	100	0	11	1			11
11-12	3 105	HARD	regulat	100	0	11				A 1
12-13	3:44	SOFE	MRE gutt	100	0	11				14
13-14	0 105	11	110	100	0	11				1/
14-15	0:04	11	11	100	0	11				31
15-16	0 05	11	11	100	0	11				11
16-17	0 03	81	11	100	0	11				11
17-18	0 :64	//	11	100	0	11				11
18-19	0 04	1/	11	100	nosamp					11
19-20					,					()
20-21										
21-22										
22-23										
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36-37										
37-38					_					
8-39								1		
9-40					a					C. C
10-41										
1-42										54
2-43										
3-44										1852 - 77
4-45										
5-46										
6-47										
7-48										
8-49										
9-50						A				
0-51								-		
1-52					-					
2-53										
3-54										
4-55										
5-56										
6-57										
7-58										
8-59										
9-60										

Rig Operator: Data Descriptions by:

Doc Engine Compressor Start: Start:



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PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION		
Client:	BH Name: USN BM17	Date: 17/0// 7018	Air Capacity (m3/m):	
Project:	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):	
Location:	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):	
X Coord: 26, 00 357	Elevation (m): 15 7	Water Rest Level (m) (After 24 Hrs):		
Y Coord: 28, 2 700 16	Bit Diameter (mm):	Water Strike (m):	1	
General Řemarks:				

		T	T		T	T	1	Т	Т	
	Penetration									
Depth	Rate	Formation	Hammer	Air Loss	Sample	Moisture	Hole	Water	Water Foam	Comments
	(min/m)	Hardness	Rate		Recovery	Condition	Collapse	Strike	Applied	Comments
						l .				
0-1	0:04	SOFO	Regulas	08	100	MOIST				
1-2	0 09	11	11	0	100	11				
				0		17				
2-3	0:11	11	1)	0	100	11				
3-4	0:19	11	0	0	100	11				
4-5	0:12	11	11	0	100	11	1			
5-6	0:31	11	77	8			1			
	0 . 31	1	no hamm	0	100	11				
6-7	0:08	11	10 hamm	05 n	OSAMI	ile				
7-8	0:04	11	I Regulax	100	0	demonste)	. 1	
8-9	0:05	11	11	100	0		1	MI	1 . 1	/
	0:07			100	0	dame	-		11/1	1
9-10	0 . 0 4	11	11	100	0	economicop	/	11	11+	
10-11	0:04	11	11	100	0	Laboratory	/	11) \/	(/)	/
11-12	0:09	11	11	100	Ø	Printeramp	/	V		/
	0:11	11	11	100	0					
12-13		1-1		100	Q	is different below				
13-14	0:18	11	11	100	0	of the last				
14-15				<i>2</i>						
15-16										
	, ,		-							
16-17			-							
17-18										
18-19		- 0								
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44-45	Α.									
						14				
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46-47										
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51-52										
52-53										
53-54				100						
54-55						(12)		X X		
55-56						710				
56-57							-			
57-58										
58-59										
59-60									_	

Rig Operator: Data Descriptions by:

Doc Engine

Start:

STOP because of big Cavity; High Risk of Rots



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PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION		
Client: ESNO m	BH Name: BJN B4 18	Date: 15/0//20/0	Air Capacity (m3/m):	
Project: Aclown 78 800	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):	
Location: ESELEN	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m)·
X Coord: 26,06622	Elevation (m):	Water Rest Level (m) (After 24 Hrs):	J J J J J J J J J J J J J J J J J J J	
Y Coord: 27; 2601.3	Bit Diameter (mm):	Water Strike (m):		

Depth	Penetration Rate (min/m)	Formation Hardness	Hammer Rate	Air Loss	Sample Recovery	Moisture Condition	Hole Collapse	Water Strike	Water Foam Applied	Comments
0-1	0.09	Soft	Logidar	090	100/0	S. Mai	1			
1-2	0.09	19	174	er	in	01				
2-3	(0) :009	17	er	6(51	tr				1 Her VI A
3-4	025	CC	11	4	11	CI				4
4-5	216	17.	1/	11	11	(1			-	Cours
5-6	0.08	carrity		80%	2010					- Corry
6-7	120	Hard	Regulat	090	100%	(1)				
7-8	1.58	11	Fr	11	10070	11				-
8-9	1.23	11	11	11		11				1/887 0
	7.07		(7	11	11				211-0019
9-10	200	7	r	16	11	BRY				Adam
10-11	4.30	· ·		cl	11	CE				00/0-1/6
11-12	73 300	(1	11	1	()	Samila		2. 1	A	
12-13	3.38-	er	11	1.1	11	DKY			,	,
13-14	3.33	e/	11	11	11	171				
14-15	3.37	10	11	- (/	11	((
15-16	3.53	((((1	1	11				f of
16-17	4-25									
17-18										1000
18-19										
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0-31				_						
1-32										
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4-35										
5-36							40			
6-37					_					
7-38										
8-39		2								
9-40	9									
0-41										
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5-46										
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3-49										
9-50								÷		
0-51										
1-52										
2-53										
3-54										
1-55										
5-56								1		
5-57										
-58										
1-59								1		
									1	

Rig Operator: TRINCE MONGWE
Data Descriptions by: TSULID CEBASA

Doc Engine

Start: Start:

Finish:

THOKA GEOSCIENCES CC

FIELD APS: STAUTS;
BONGAN; GADEBE
Sello MGWENY &
lufo fé MPAKUMPAKU
HLy LANS MALY LÉKÉ

5 26.00527° E 28.26726° Ely 1572mg.



Percussion Borehole Drilling Records

Version Date: May 2015

Head Office: 35 Woltemade Street, Culemborg Park, Randfontein, 1759. Postal Address: P O Box 1910, Silverton, 0127 Tel/Cell: +27 82 932 8465 Fax: +27 86 516 8363 Email: thokageo@gmail.com

PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION	
Client: ESKO n	BH Name: ESN BH 19	Date: 10/01/2018	Air Capacity (m3/m):
Project: Dolow178 STUBY	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):
Location: Esselen	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):
X Coord: 26.00 527	Elevation (m): 15.9% ~	Water Rest Level (m) (After 24 Hrs):	, and a second s
Y Coord: 28-26726°	Bit Diameter (mm):	Water Strike (m):	
General Remarks:			

Depth	Penetration Rate (min/m)	Formation Hardness	Hammer Rate	Air Loss	Sample Recovery	Moisture Condition	Hole Collapse	Water Strike	Water Foam Applied	Comments
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PRINCE MONGWE Rig Operator:
Data Descriptions by: TSELISO GBASA

Doc Engine Compresso Start:

Finish: Finish:

THOKA GEOSCIENCES CC

FIELD ASSISTANTI:
SELLO MENENYE
BONGANI SHOEBE
LYFEFO MPARUM PAKY
HU CANI MALU GOKE



Head Office: 35 Woltemade Street, Culemborg Park, Randfontein, 1759. Postal Address: P O Box 1910, Silverton, 0127 Tel/Cell: +27 82 932 8465 Fax: +27 86 516 8363 Email: thokageo@gmail.com

PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION		
Client: Eston	BH Name: EJN BH 20	Date: 15/01/3018	Air Capacity (m3/n	n):
Project: Oplow TO STUBY	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):	
ocation: Esseten	Casing Installed (m):	Blow Yield (L/s):	Casing Permanent	tly Damaged (m):
(Coord: 26,00558	Elevation (m):	Water Rest Level (m) (After 24 Hrs):		
Y Coord: 25 27080	Bit Diameter (mm):	Water Strike (m):	j. 1)	
General Remarks:				-

Depth	Penetration Rate (min/m)	Formation Hardness	Hammer Rate	Air Loss	Sample Recovery	Moisture Condition	Hole Collapse	Water Strike	Water Foam Applied	Comments
0-1	0.11	Saft	Render	0%	100%	g.moog				011
-2	0.10	14	110	H	111	11				CHEST 1
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-6	Ph.40	1/	rf.	11	10	11			3	Man
-7	11.59	11	1.1	, r	11	1/			000	- West
-8	13.516	11	11	LL		DRY			*	
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Start:

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THOKA GEOSCIENCES CC

FIELD ASSISTANTS
BONSAN/ SADEBE
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HYLAN, MARY GRE



Head Office: 35 Woltemade Street, Culemborg Park, Randfontein, 1759, Postal Address: P O Box 1910, Silverton, 0127 Tel/Cell: +27 82 932 8465 Fax: +27 86 516 8363 Email: thokageo@gmail.com

PROJECT INFORMATION	BOREHOLE INFORMATION	HYDROLOGY INFORMATION		
Client:	BH Name: BSN BH21	Date: 1 7 10117018	Air Capacity (m3/m):	
Project:	Total Depth (m):	Air Pressure (kPa):	Casing In & Out (m):	
Location:	Casing Installed (m):	Blow Yield (L/s):	Casing Permanently Damaged (m):	
X Coord: 2161003990	Elevation (m): 15 + 3 m	Water Rest Level (m) (After 24 Hrs):		
Y Coord: 22, 27073°	Bit Diameter (mm):	Water Strike (m):	•	
General Remarks:				

Depth	Penetration Rate (min/m)	Formation Hardness	Hammer Rate	Air Loss	Sample Recovery	Moisture Condition	Hole Collapse	Water Strike	Water Foam Applied	Comments
0-1	0:18	SOFt	Requis	08	100%	Moist	-			
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Rig Operator: Data Descriptions by:

Doc Engine Compressor Start: Start:

Appendix V

Gravity Survey Results



Geophysical Services Reg: 2016 / 324488 / 07 106 Clearwaters Cove Haenertsburg 0730 Limpopo RSA

gavin@geofocus.co.za (083 478 4345) bjorn@geofocus.co.za (072 086 9829)

011 412 1449

079 327 5439

Attention: Melusi Mashego

Tel:

Cell:

05 February 2018

Thoka Geosciences CC

35 Woltemade Str Culemborg Park

Randfontein, 1759

Gauteng

RSA **E-mail**: melusim@thokageo.co.za

Esellen Substation – Gravity Survey

1. General

A gravity survey, in support of a dolomite-stability study, was conducted over roughly 12.5 ha surrounding Escom's Esselen Sub-Station, across the R21 from Tembisa, on 10 October 2017. The area was covered with a 15m grid comprising 622 stations.

2. Methodology

Gravity was observed with a Scintrex CG5 gravimeter whilst a Javad DGPS recorded station locations. Gravity data processing procedures commonly used for dolomite studies were applied, firstly reducing the data to relative Bouguer values by applying elevation, terrain and Bouguer corrections. Secondly, an estimation of the regional gravity field (slope), calculated either through linear regression or a grid trend, is removed from the relative Bouguer gravity to produce an uncorrected or provisional residual gravity map emphasizing local changes.

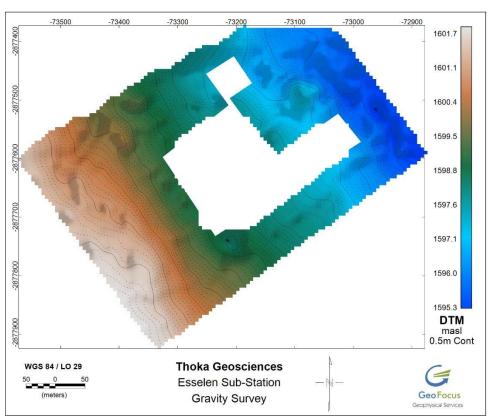
The final residual gravity map is produced by correcting for bedrock head intersections acquired during outcrop mapping, pitting and/or drilling. This usually entails a base shift (adjustment by a constant) so that the residual gravity values agree on average with bedrock depths. This is done by shifting the residual Bouguer highs to represent outcrop, where present, as zero (0) mGal. Otherwise drill locations where bedrock was intersected are adjusted so that the corresponding residual Bouguer value approximates the ratio of -1mGal = 48m. In some cases, notably small surveys, bedrock head intersections can provide the necessary spatial correlation needed to recalculate the regional gravity field in order to achieve a better fit.

3. Results

The median elevation of the survey area is approximately 1,599 masl (relative) sloping southwest to northeast by roughly 7 m (Figure 1). A 1st-order approximation of the regional gravity field was removed from the Bouguer anomaly data (Figure 2) resulting in a residual gravity dataset with peak-to-peak distribution of approximately 0.4 mGal (Figure 3).

4. Residual Gravity

Corrections were applied to the residual gravity data based on a set of nineteen (19) boreholes drilled across the site (Figure 3). It is understood that rehabilitated sinkholes exist within the surveyed area which are likely underlain by concrete and other foreign rock material. The broad gravity high which spans most of the surveyed area, and surrounds the existing substation, is shown to consist of shallow (3m to 4m) dolomite bedrock head as intersected by boreholes BH20 and BH6. Subtle changes in the residual





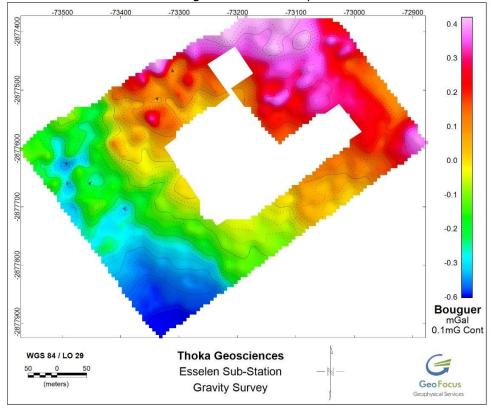


Figure 2: Relative Bouguer Map

gravity, within the broader high, correspond with variations in weathering as noted by boreholes BH9, 11, 13 & 18 - 19 which intersected competent bedrock formation at between 8m and 11m deep. Bedrock deepens toward the northeast, coincident with a broader gravity low, where boreholes BH7 - 8, 15 & 21 intersected bedrock head between 13m and 18m deep. In most cases small cavities were intersected which caused sample loss. Boreholes 16 & 17, drilled on the gravity gradient, intersected fairly large cavities and were terminated at 19m and 14m respectively; both of these locations fall on subtle and small lows originating from the larger gravity low. The gravity low toward the southwest is more irregular and appears to be comprised of two lows, located in the northwest and southwest, connected by narrow, rectilinear lows. This is supported by boreholes BH1 & 3 which intersected bedrock at 12m and 18m respectively while boreholes BH2 & 4 were terminated after intersecting fairly large voids. Although boreholes BH5 & 12 showed bedrock to be around 5m depth, the gravity suggests they may have been drilled on the edge of a slot. Overall the drilling results show the gravity to be a fair reflection of bedrock topography.

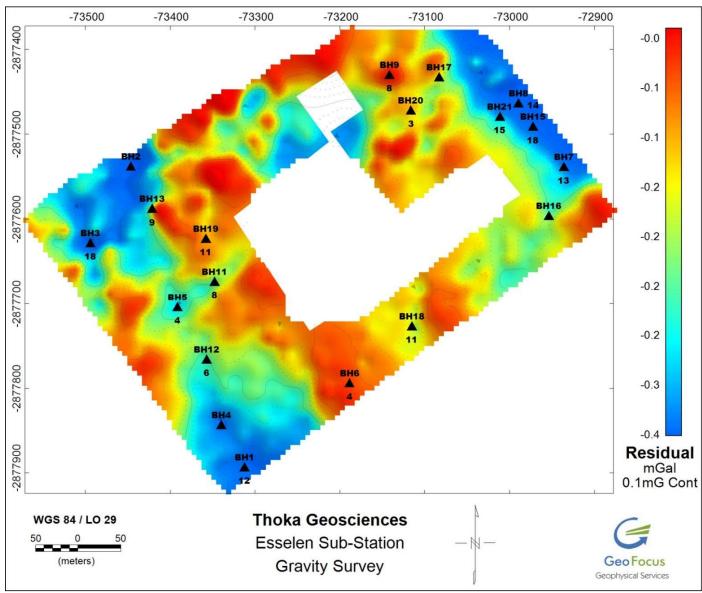


Figure 3: Residual gravity (0.1mGal contours) & drilling results

Appendix VI

Council for Geosciences Review

280 Pretoria Street, Silverton, Pretoria Private Bag X112, Pretoria 0001, South Africa

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Our Reference: F5281.1

Eskom Esselen Substation, Esselen Park, Thembisa

Your Reference: 380-02-2018 Enquiries: S Ngubelanga

> Tel: 012 841 1140/ 033 345 6265/6

Email: sngubelanga@geoscience.org.za

No. of Pages: 4

14 March 2018

Ekurhuleni Metropolitan Municipality P O Box 13 Kempton Park 1620

ATTENTION: Mr. Pilusa Mashamaite

By Email:

Pilusa.Mashamaite@ekurhuleni.gov.za

Dear Sir,

ESKOM ESSELEN SUBSTATION IN ESSELEN PARK, THEMBISA

The firm, Thoka Geosciences CC (TG) submitted their report: "Dolomite Stability Investigation Report: Eskom Esselen Substation, Esselen, Thembisa, Gauteng, South Africa", dated February 2018 to this office for comment on behalf of their client, Eskom SOC Ltd, on 13 February 2018. This office acts as an agent to state authorities in reviewing dolomite stability investigations on their behalf.

The purpose of the current investigation was to assess the exposure and vulnerability of the site to dolomite geo-hazards which have historically manifest into sinkholes and surface subsidence. Esselen Substation covers a surface area of about 19 hectares measuring from the outer perimeter fence. This investigation is solely for the area bound by the outer parameter fence and the HV Plant, which covers about 12.5 hectares of the total 19 hectares that makes up Eskom Esselen Substation. The site is located in Esselen Park, Tembisa bounded by the R21 Highway on the west and the R25 on the south.

The proposed land use is a substation, C3 type development in terms of the SANS 1936:2012.

The following is noted from TG's report:

- 1) The regional geology of the area is underlain by the Malmani Subgroup dolomitic rocks of the Chuniespoort Group (1: 250 000 Geological Series, 2628 East Rand). The assessment of the geology within the site boundaries indicated outcropping dolomite at isolated parts of the site interpreted to be panicles of dolomite.
- 2) In Section 3.1 of the report, TG indicates that none of the drilled borehole at site encountered groundwater. The deepest borehole drilled at site reached depth of 24 m from the surface. This infers a deeper groundwater table at the site and that sinkhole or subsidence associated with groundwater drawdown is unlikely but rather ingress from ponding / percolating surface water is the main trigger for sinkhole formation at the site.

TG further states that the assessment of regional groundwater indicates groundwater levels to be at maximum depth of about 30 m from the surface for monitoring station A2N0627. The site is located within the Sterkfontein Dolomite Compartment in the A21A Groundwater Management Unit in the Crocodile West and Marico Water Management Area. The hydrogeological properties of the compartment are characterized by the karst nature of dolomite aquifers.

- 3) A gravity survey was conducted on a 15 m grid spacing by GeoFocus Geophysical Services in October 2017.
- 4) A total of twenty one (21) percussion boreholes were drilled during the current study. These boreholes varied in depth between 9 m and 24 m below surface and they generally intersected:
 - Transported soils (colluvium) between surface and 8 m depths,
 - Highly weathered chert and dolomite between 1 m and 5 m depths,
 - Cavities between 3 m and 9 m depths.
 - Weathered to solid rock (chert & dolomite) between 2 m and 19 m depths,
- 5) Based on the available information, TG has classified the site as having the following Inherent Hazard Class (IHC) zones
 - IHC 3/4/6/7 with a D3 dolomite area designation.
 - IHC 8 with a D4 dolomite area designation.
- 6) TG has made recommendations in section 6 and conclusions in section 7 of the report and the following is noted:
 - All new structures developed should be designed to span at least 5 15 m loss of support. An engineered soil raft / raft should be used in areas of shallow dolomite bedrock. This mattress has the dual purpose of improving founding conditions.
 - According to SANS 1936, in addition to the above mentioned precautionary measures, sites which fall under D4 dolomite land need to adhere to site-specific precautionary measures.
 - A groundwater monitoring station at the site is recommended as part of a risk management facility.

- The site falls under C3 type development, thus portions of the site are regarded to be conditionally developable with footprint requirements up until IHC 6.
- A crucial requirement of development on dolomitic lands is the establishments and implementation of a dolomite risk management strategy (DRMS) by the developer in terms of SANS 1936:2012.

This office would like to comment as follows:

- a) This office is broadly in agreement with the hazard assessment and zonation of the site. TG has classified the site as follows:
 - IHC 3/4/6/7 with a D3 dolomite area designation and this is supported.
 - IHC 8 with a D4 dolomite area designation and this is supported.
- b) TG has indicated that the proposed land use is considered to be a electrical substation, (C3) type development. According to Table 2 of SANS 1936-1:2012, C3 type developments are permissible up to IHC 6 land, subject to D3 precautionary measures and footprint investigations:
 - This office confirms that the drilling of twenty one (21) boreholes is considered adequate and meets the minimum drilling requirements as stipulated in SANS 1936:2012.
 - This office confirms that the geological conditions as revealed by drilling results are considered to be highly variable and generally high hazard in water ingress scenario.
 - Dolomite instability events (sinkholes) have occurred in the vicinity of the substation.
- c) Given that the site has revealed patches of IHC 7/8 conditions within generally good conditions (3-6), any new development may only be considered provided that additional infill investigations (footprint) and stringent precautionary measures are adhered to, with appropriate foundation designs or a "D4-Route" development process would have to be followed, including a review of all information by the Level 4 Geo-professional as per SANS 1936:2012.
- d) The conclusions in recommendations in section 6 and conclusions in section 7 of the report are generally supported

Therefore this office confirms the provisional support of the proposed Eskom Eselen substation, (C3) type development on Essellen Park in Thembisa, subject to the points above and the following:

- e) Additional investigations FPI), must be conducted and the results thereof, submitted to this office for further comments, prior to any construction on this site.
- f) A certified site development plan should be submitted to this office for co-signing.

- g) TG or the professional team involved are to certify that additional investigations, the development proposal and precautionary measures are in accordance with SANS 1936:2012 requirements.
- h) A site specific Dolomite Risk Management Plan in accordance with SANS 1936-4:2012 must be compiled and implemented for the site. The owners/responsible persons must be made aware of the risks involved in building on dolomite, and be informed about how to be vigilant and act pro-actively by applying sound water management principles.
- The Local Authority must implement a risk management system. Commenting on the suitability of sites within its jurisdiction is based on the premise that this system will be implemented.

This letter reflects the Council for Geoscience's view and approach to development on dolomite at this time, as reflected by the above date. These comments may not be viewed as open-ended. If a property changes ownership or land-use changes are made, the comment may in part or wholly no longer apply. This Office should be informed of such changes and the Competent Person responsible for the dolomite stability investigation should be given the opportunity to indicate the influence such changes could have on the overall stability.

If you have any further queries, please do not hesitate to contact this office.

OR

Yours faithfully.

S NGUBELANGA Engineering Geologist

Eskom Esselen Substation, Esselen Park, Thembisa (F5281.1)

CC: Thoka Geosciences CC

ATTENTION: Mr. Tshwaro Mogadime,

By email: thokageo@gmail.com